



FLOOD RISK ASSESSMENT

FOR

**FORMER HOPE HIGH SCHOOL SITE,
SKELMERSDALE**

ON BEHALF OF

**TAWD VALLEY DEVELOPMENTS LTD/
WEST LANCASHIRE BOROUGH COUNCIL/
LANCASHIRE COUNTY COUNCIL**



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Flood Risk Assessment for Former Hope High School Site, Skelmersdale

NW134/01/2r3

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1.0 INTRODUCTION

- 1.1 Tawd Valley Development Ltd are proposing a residential development on the site of the Former Hope High School and adjacent additional land, off Tanhouse Road, Skelmersdale. The site is within Flood Zone 1, but the site area is greater than 1 hectare, and a Flood Risk Assessment is therefore required to support the planning application.
- 1.2 It is within the general development strategy of the country for development in areas where there is a risk of flooding to be assessed to avoid unacceptable impacts upon the proposed development itself and flood risk elsewhere. Under the National Planning Policy Framework (NPPF) (Communities and Local Government; June 2019) and the associated Planning Practice Guidance (PPG) (Communities and Local Government; October 2019), a Flood Risk Assessment Report should be prepared to consider the risk of flooding from all potential sources and make recommendations for any necessary flood mitigation measures.
- 1.3 ARP Associates have been appointed to implement appropriate consultations, carry out an assessment of the site, and prepare a Flood Risk Assessment Report, in accordance with the requirements of NPPF, to satisfy the requirements of the Planning Authority.
- 1.4 The consultations and site appraisal for this assessment were carried out between January 2020 and May 2022.
- 1.5 The report has been initially prepared for the use and reliance of Tawd Valley Developments Ltd, West Lancashire Borough Council, and Lancashire County Council only. The report shall not be relied upon or transferred to any other parties without the written agreement of ARP Associates. For the avoidance of any doubt, where ARP Associates enters into a letter of reliance for the benefit of a third party, that third party will be permitted to rely on the report. No responsibility will be accepted where this report is used, either in its entirety or in part, by any other party without ARP Associates consent.
- 1.6 Attention is drawn to the requirements of the Construction Design and Management Regulations 2015, and in particular, the duties and obligations of the Client.

2.0 SITE DESCRIPTION

General

- 2.1 The site, which is centred on Ordnance Survey Grid Reference 350290, 405620, is located off Tanhouse Road, Skelmersdale. The nearest postcode is WN8 6BJ. A site location plan and aerial image is presented in Appendix A.
- 2.2 The site is an irregular shaped piece of land extending to an area of approximately 2.8 hectares (ha), with approximate dimensions of 290m (north - south) by 170m (east - west).
- 2.3 This assessment is informed by desktop study, topographical and drainage surveys, and by three site walkover visits undertaken by ARP in January 2020, September 2021, and November 2021.

Current Use

- 2.4 The site is currently occupied by a former school building, with associated hardstanding and landscaped areas. The existing building, located in the western area of the site, is currently disused and unoccupied. A car park is present to the north of the school building, and bitmac hardstanding areas used for sports activities are present to the south and east of the building. A small electricity sub-station is also present at the southwestern corner of the school.
- 2.5 The southeast of the site comprises relatively flat grassland which, when compared to the surrounding areas, appears to have been levelled at some time in the past. This area of the site is bound by two watercourses (discussed below) with areas of significant vegetation around the perimeter, including around the watercourse channels.
- 2.6 The north of the site is an area of unused and undulating ground, which was heavily vegetated before clearance took place in late 2021.
- 2.7 The general wider area beyond the site falls from the northeast towards the southwest.

Boundaries and Surrounding Land Use

- 2.8 The site is bounded to the north, east, south and west by residential properties as part of a larger housing estate. Historical aerial images show the surrounding housing estate to have been developed post-2000. To the east and south, beyond the residential properties, are 'A' roads as part of the infrastructure network of Skelmersdale.

Topography

- 2.9 A topographical survey of the site was undertaken in August 2020, with updates made in January 2022, and a copy of the survey drawing is included in Appendix B for reference. It can be seen that the majority of the site generally falls from the north to the southwest, although the topography within the site is variable and undulating.
- 2.10 Levels to the northernmost tip of the site are of the order of 104.5mAOD, which is coincident with the adjacent road level on Mercury Way to the northeast. The levels within the northern sector of the site fall towards the southwest, to a level of approximately 99.5mAOD at the north of the bitmac surfaced sports pitch. The bitmac surfaced pitch falls slightly to a level of around 98.8mAOD in its southern corner. Directly south of this, levels fall steeply down a small embankment, falling to around 98.0mAOD just north of the existing building.
- 2.11 Levels around the existing building and hard paved areas vary from around 98.0mAOD in the north, to 96.2mAOD in the south. Levels continue to fall south through the dense vegetation, towards a level of around 95.0mAOD at Mercury Way.
- 2.12 The majority of the eastern section of the site is relatively flat, falling from north to south from around 98.0mAOD to 97.0mAOD. At the north of this area levels rise more steeply towards the northern boundary, which lies at approximately 101.5mAOD. Levels also fall steeply around the perimeter of this area, into the watercourse channels which surround it (discussed further below). The south of this area falls to a low point of approximately 91.2mAOD at the downstream end of the watercourse channel.

Existing Drainage

- 2.13 United Utilities public sewer records (Appendix E) indicate no public sewers within the site itself. Separate systems of surface water and foul public sewers are located in the residential areas surrounding the site. The records indicate that Within Mercury Way, to the south of the site, there is a 225mm diameter foul public sewer and a 1350mm diameter surface water public sewer.
- 2.14 A survey of the existing drainage systems on site was undertaken by IWJS in November 2021. A copy of the drainage survey report and plans is included in Appendix C for reference.
- 2.15 The survey confirmed that both surface water and foul drainage from the existing building and its surrounding hardstanding (including car park and playground) is drained by a combined system of private drainage, which discharges from the west of the site. It is likely that the discharge is made to a public sewer at the rear of the properties to the west of the site, although the sewer does not appear on the United Utilities records.
- 2.16 The survey identified two surface water outfalls to the watercourse on site. Both outfall pipes appeared to flow from direction of the school, however as above these do not appear to be serving the school building or playground. The southernmost outfall pipe was found to be collapsed just upstream of the outfall, whilst the northernmost outfall pipe could be traced towards the north of the site however it too was found to be collapsed at a point close to the northeast corner of the existing building. It is likely that these pipes are historic land drainage systems or systems which served the former school buildings which were located to the northwest.
- 2.17 The uneven northern area of the site, and the grassed eastern are not believed to be served by drainage systems, although the presence of land drainage cannot be ruled out. Runoff from these areas may infiltrate to a degree, but it is likely that the majority simply run towards the existing drainage system serving the building and hardstanding, or into the watercourses within the site.

Hydrology

- 2.18 Two un-named Ordinary Watercourses are present on the site, one each side of the playing field in the east of the site, both flowing towards the south and both set in relatively deep channels. In the south of the site the watercourses confluence before entering a 750mm diameter culvert which flows south towards Mercury Way. It is noted however that the culvert is approximately 3.5m below the level of Mercury Way, therefore it is likely that it passes under Mercury Way and towards the underpass beneath Grimshaw Road to the south.
- 2.19 There is an existing crossing of the westernmost watercourse channel, which facilitates access from the school playground on the west into the eastern area of the site. The crossing is crudely constructed with a stepped arrangement of kerbstones used to form the headwalls. It is assumed that the watercourse is culverted beneath the crossing, however no culvert is visible at present and is likely to be silted up/buried.
- 2.20 At the head of each channel within the northeast of the site, is an incoming pipe. A 150mm diameter pipe enters the head of the westernmost channel, and a 300mm diameter pipe enters the head of the easternmost channel. The pipes appear to enter from the direction of the adjacent housing estates, although it is noted that the surrounding plot levels are significantly higher than the level of the pipes.
- 2.21 Attempts were made to survey the two incoming pipes during the drainage survey in November 2021, however the 150mm pipe became a 100mm pipe after a short distance and could not be surveyed, whereas the 300mm pipe was found to be highly silted approximately 26m upstream of the site.
- 2.22 Review of available historic mapping confirms that both watercourses once extended upstream into what was then open fields. However, the upstream extent does not appear to be significant, with both beginning less than 250m outside of the site.
- 2.23 The watercourses were observed to have very little baseflow during two site visits undertaken by ARP in January 2020 and September 2021, and when the topographical survey and update

survey were undertaken in August 2020 and January 2022. A steady flow was observed in the watercourses during the site visit undertaken in November 2021, but this was on a day of very heavy and prolonged rainfall.

- 2.24 It is considered likely that the incoming pipes are land drains installed along the line of the upstream watercourse sections before development took place, and that they are unlikely to have a significant upstream catchment.
- 2.25 The 750mm diameter culvert which flows out of the site after the watercourses confluence in the southeast, was also surveyed during the drainage survey in November 2021. The survey reached approximately 34m downstream of the site before being abandoned due to silt and debris. The section of culvert which was surveyed was in good condition.

3.0 CONSULTATION

Environment Agency

3.1 Consultation was undertaken with the Environment Agency, and a copy of its response, reference CL157879BL dated 21st January 2020, is presented in Appendix D for reference. Publicly available Environment Agency data has also been reviewed for reference, from which extracts are also included in Appendix D for reference.

3.2 The Environment Agency Flood Map for Planning shows areas of land considered to be at risk of flooding from rivers or the sea. These areas do not take into account defences, as water can overtop them or the defences can fail in extreme conditions. The Flood Zone classifications are:-

Flood Zone 1 - "Low Probability" is assessed as having a less than 1 in 1,000 annual probability of river or sea flooding in any year (less than 0.1%).

Flood Zone 2 - "Medium Probability" is assessed as having between a 1 in 100 and 1 in 1,000 annual probability of river flooding in any year (1% - 0.1%) and between a 1 in 200 and 1 in 1,000 annual probability of flooding from the sea (0.5% - 0.1%).

Flood Zone 3 - "High Probability" is assessed as having a 1 in 100 or greater annual probability of river flooding in any year (greater than 1%) and a 1 in 200 chance or greater annual probability of flooding from the sea (less than 0.5%).

3.3 The Flood Map for Planning and the Environment Agency response confirms that the site is located within Flood Zone 1. However, it is noted that the Flood Map for Planning does not show the risk associated with watercourses which have a small catchment.

3.4 The Environment Agency confirmed that it does not hold any flood model information relevant to the site.

3.5 The Environment Agency Risk of Flooding from Surface Water map shows areas which may be at risk of flooding from surface water flow paths or surface water ponding during a significant rainfall event. The classifications of risk for surface water flooding are as follows:-

Very low - This area has a chance of flooding of less than 1 in 1000 (0.1%) in any given year.

Low - This area has a chance of flooding between 1 in 1000 (0.1%) and 1 in 100 (1%) in any given year.

Medium - This area has a chance of flooding between 1 in 100 (1%) and 1 in 30 (3.3%) in any given year.

High - This area has a chance of flooding greater than 1 in 30 in any given year (annual probability of flooding 3.3%).

3.6 The Environment Agency Risk of Flooding from Surface Water map suggests that the majority of the site is at a 'very low' risk of flooding from surface water. The mapping suggests a potential area of ponding within the northern section of the site in the 'low' and 'medium' risk scenarios. An area of ponding on the eastern side of the existing building in the 'low' risk scenario is also suggested.

3.7 A more significant area of potential surface water flooding in the 'high', 'medium' and 'low' risk scenarios is suggested along the existing watercourse corridors and around the confluence of the two watercourses in the south of the site, upstream of the culvert which conveys flow out of the site. In the 'low' risk scenario the mapping indicates an overland flow path from this area, into Mercury Way to the south.

- 3.8 Outside of the site, a number of the surrounding roads are shown to be at risk of flooding, although this is predominantly in the 'low' risk scenario.
- 3.9 The Environment Agency's online Flood Risk from Reservoirs map has also been reviewed as part of this study. This mapping shows the expected inundation area should a reservoir fail and release its capacity. The site is not located in an area shown to be at risk of flooding from this source.

Water Authority

- 3.10 Consultation was undertaken with United Utilities, which is the Water Authority for this area, and a copy of its response, reference 4200030297 dated 20th February 2020, is presented in Appendix E for reference.
- 3.11 United Utilities confirmed that foul water can discharge to the local public foul sewer network at an unrestricted rate. A specific point of connection was not mandated.
- 3.12 In respect of surface water, United Utilities noted that consideration should firstly be given to discharge to soakaway, and watercourse, in that priority order, before connection to sewers will be considered. In the event that a discharge to sewer is required, surface water can discharge to the local public surface water sewer network at a rate to be agreed with the Lead Local Flood Authority, but not exceeding 15l/s.

Local Authority

- 3.13 Consultation with West Lancashire Borough Council's Technical Service team was undertaken in January 2020. The engineer confirmed no knowledge of drainage or flood risk issues regarding this site.
- 3.14 Attention was also drawn to the local guidance document 'Planning Applications – Drainage, Flood Risk and Sustainability' which sets out expected principles for surface water drainage in new development. In particular it is noted that an additional allowance of 10% on proposed

impermeable area is required to account for the potential impacts of urban creep, and that a climate change allowance of an additional 40% on rainfall intensity is preferred in calculations.

Lead Local Flood Authority

3.15 Lancashire County Council, which is the Lead Local Flood Authority (LLFA) for this area, has not been consulted at this stage, however, based on recent experience it is anticipated that the LLFA will have the following general requirements for surface water management:

- The drainage strategy shall be based upon the sustainable drainage principles and requirements set out in the National Planning Policy Framework, Planning Practice Guidance and Defra Technical Standards for Sustainable Drainage Systems. Calculations shall be provided for peak flow control and volume control for the 1 in 1 year, 1 in 30 year and 1 in 100 year plus 40% climate change allowance events.
- An allowance of plus 10% on impermeable areas will be required to allow for urban creep.
- Flood water exceedance routes will need to be considered in accordance with Defra Technical Standards for Sustainable Drainage Systems.
- Finished Floor Levels should be a minimum 150mm above external levels.
- Consideration will be required of measures taken to manage the quality of the surface water runoff to prevent pollution, protect groundwater and surface waters, and delivers suitably clean water to sustainable drainage components
- A Construction Phase Surface Water Management Plan will be required to detail how surface water and pollution prevention will be managed during each construction phase
- An Operation and Maintenance Plan will be required, including arrangements for adoption by an appropriate public body or statutory undertaker, and/or management and maintenance by a Management Company and any means of access for maintenance and easements, where applicable.

4.0 MATERIAL CONSIDERATION IN RESPECT OF NPPF AND PPG

Flood Classification

- 4.1 The Environment Agency Flood Map for Planning has identified that the site falls within land assessed as having less than a 1 in 1,000 (0.1%) annual probability of river flooding. In accordance with Table 1 of the PPG, the site falls within Flood Zone 1, "low probability".

Proposed End Use

- 4.2 The development proposals for the site are for the construction of 54 residential dwellings, with associated access, car parking, and open space. The proposed site access is from Tanhouse Road to the west. The development proposals are illustrated in the plan in Appendix F.
- 4.3 When applying Table 2 of the PPG, the flood risk vulnerability classification shows that the proposed end use is considered to be "More Vulnerable" development under the general designation "Buildings used for dwelling houses".
- 4.4 As set out in Section 2 and indicated on the plan in Appendix C, there are two Ordinary Watercourse within the east of the site, which confluence towards the south and flow out of the site by means of a culvert flowing in a southerly direction. The watercourses will remain in situ within the proposed development.
- 4.5 To allow for future maintenance access, the development layout includes a minimum 5m buffer between proposed buildings and the top of the bank of these watercourses. With the exception of one proposed plot near the head of the westernmost channel, land within the 5m buffer will not be located within individual property boundaries, but will be located within public open space, shared private access roads, or proposed adoptable carriageways, thus preserving maintenance access.

- 4.6 As part of the proposed development, the existing crossing of the westernmost channel will be removed, and the channel reinstated. A new adoptable road crossing will be constructed over the westernmost channel to facilitate access to the eastern part of the site.
- 4.7 Improvements will also be made to the downstream culvert entrance by installing an appropriately designed trash screen, to reduce the risk of blockage and to make clearance easier.

Sequential Test

- 4.8 As set out in the NPPF, the aim of the Sequential Test is to steer new development to areas with the lowest probability of flooding. As the site is located within Flood Zone 1, there is no requirement for application of the Sequential Test.

Exception Test

- 4.9 When the proposed development is evaluated in accordance with Table 3 of the PPG, is determined that there is no requirement to apply the Exception Test to “More Vulnerable” development in Flood Zone 1.

Flood Sources

- 4.10 Flooding from the Sea - The site is not located near enough to the sea to be at risk of flooding from this source.
- 4.11 Flooding from Rivers - The site is located in Flood Zone 1, suggesting a ‘low’ probability of flooding from rivers, however this mapping does not represent flooding associated with watercourses which have a small upstream catchment (normally less than 3km²). On this basis, the potential risk of flooding associated with the Ordinary Watercourses within the site requires further consideration.

- 4.12 As set out in Section 2, review of historic mapping and survey of the incoming pipes at the head of the two watercourses suggests that they are fed by land drainage pipes beneath the adjacent residential areas, but with no significant upstream catchment. Two outfalls to the easternmost channel were identified but could not be traced a significant distance upstream due to collapses, and are likely to be the remnants of abandoned drainage systems or land drains. At the downstream end of the site, the two watercourses confluence and outfall via a 750mm diameter culvert which appears to be in good condition.
- 4.13 It is noted that consultation with West Lancashire Borough Council Technical Services team did not identify any known reports of fluvial flooding having occurred from the watercourses.
- 4.14 As hydraulic modelling of these small watercourses is not available, a simplified but conservative assessment of inflow against outflow has been undertaken. The assessment is based on a conservative assumption that the incoming pipes discharge flows at pipe full capacity, and are at a steep gradient of 1 in 10. Flows have been calculated on this basis using the Colebrook-White formula. Greenfield inflows from within the undeveloped areas of the site have been estimated by determining the approximate greenfield catchment size from the topographical survey, and assuming a runoff rate of 14.60l/s/ha, which is the 1 in 100 year greenfield runoff rate as derived using the IoH124 method (Appendix G). The capacity of the 750mm culvert which conveys flows out of the site has also been estimated using the Colebrook-White formula, but a conservative estimate of a 1 in 500 gradient has been applied to this pipe.
- 4.15 The assessment is set out as follows:

Inflow to western channel:

- 100mm dia. pipe at head of channel - pipe full capacity at gradient of 1 in 10 = 19.27l/s
- 150mm dia. pipe from west (1) - pipe full capacity at gradient of 1 in 10 = 56.55l/s
- 150mm dia. pipe from west (2) - pipe full capacity at gradient of 1 in 10 = 56.55l/s
- Greenfield Contribution – Estimated catchment size 1.85ha - Q100 = 27.01l/s

Inflow to eastern channel:

- 300mm dia. pipe at head of channel - pipe full capacity at gradient of 1 in 10 = 353.01l/s
- Greenfield Contribution – Estimated catchment size 0.86ha - Q100 = 12.56l/s

Estimated maximum total Inflow to watercourses in 100 year rainfall event = 524.95l/s

750mm dia. downstream culvert - pipe full capacity at gradient of 1 in 500 = 547.82l/s

- 4.16 Based on the above simplified assessment, it is expected that the downstream culvert would have adequate capacity to convey the flows which enter the watercourses. On this basis the risk of fluvial flooding is considered to be low.
- 4.17 It should be noted that the pipe which conveys flow beneath existing crossing point on the westernmost watercourse cannot currently be seen, and is assumed to be buried below silt. However, this crossing and culvert will be removed as part of the development and the channel reinstated in open section. A new culvert-based road crossing of the watercourse will be provided at a location slightly further north, and this will be designed to adequately convey the expected flows without restriction (discussed further below). On this basis, the current lack of conveyance does not pose a risk to the proposed development, and the proposals will provide betterment in terms of watercourse conveyance in this area.
- 4.18 Notwithstanding the above, there remains a residual risk of flooding in the event that the downstream culvert becomes blocked, or if the flows in the watercourse exceed the capacity of the downstream culvert during a catastrophic event. Risk of blockage can be mitigated to a degree by undertaking regular inspection and maintenance, and by installing a trash screen/grille across the culvert entrance. However, in the 'worst-case' scenario, where the culvert is overwhelmed cannot and water levels rise to the top of the banks, it can be seen from the topographical survey that flood levels would not be able to exceed a level of approximately 95.0mAOD before overtopping the land to the south and flowing out over Mercury Way.

- 4.19 The Environment Agency Risk of Flooding from Surface Water mapping can also sometimes provide a reasonable representation of flood risk from small watercourses. However, it must be recognised that the Environment Agency Flood Map for Surface Water is based on available 'LiDAR' ground level data which does not include representation of hydraulic features such as culverts on watercourses. On this basis, it could be considered that the Risk of Flooding from Surface Water map provides a reasonable indication of the residual risk scenario when the downstream culvert is blocked.
- 4.20 It can be seen that the mapping indicates a significant potential risk of flooding in the 'high', 'medium' and 'low' risk scenarios around the confluence of the two watercourses in the south of the site, upstream of the culvert. In the 'low' risk scenario (which is representative of a rainfall event between 1 in 100 year return period and 10000 year return period, the mapping indicates an overland flow path from this area, into Mercury Way to the south. Comparison of the Risk of Flooding from Surface Water flood extents with the topographical survey of the site confirm that the maximum flood level indicated is of the order of 95.0mAOD. This supports the assessment set out above.
- 4.21 Residual fluvial flood risk is therefore considered to be limited to flooding up to a level of approximately 95.0mAOD, and further measures to mitigate this residual risk are set out below.
- 4.22 Flooding from Land - The Environment Agency Risk of Flooding from Surface Water map suggests that the majority of the site is at 'very low' risk of pluvial flooding. It must be recognised that the Environment Agency Risk of Flooding from Surface Water mapping is based on a generalised and relatively 'broad-brush' method, and is therefore indicative only. Although some allowance is made for the presence of drainage systems in urban areas, the full detail of existing drainage systems is not incorporated in the model.
- 4.23 The most significant area of potential flooding suggested by the Risk of Flooding from Surface Water map is that which occurs around the existing watercourses in the east of the site. The risk of flooding associated with these features has been assessed in detail in the above section on Flooding from Watercourses, and does not require further consideration.

- 4.24 In addition to the above, the Risk of Flooding from Surface Water mapping suggests some isolated areas of potential overland flow and ponding within and close to the site which warrant further consideration:
- 4.24.1 The mapping suggests a potential area of ponding within the northern section of the site in the 'low' and 'medium' risk scenarios, and an area of ponding on the eastern side of the existing building in the 'low' risk scenario. These areas are representative of existing low points where water would be expected to pond as a result of a large rainfall event. The proposed development of the site would be expected to eliminate the risk in these areas through appropriate level design and the provision of a new surface water drainage system.
- 4.24.2 The mapping suggests a potential risk of flooding within a number of the roads surrounding the site. However, this is predominantly shown to occur in the 'low' risk scenario, which represents extreme rainfall events between 1 in 100 year return period and 1 in 1000 year return period, and the mapping does not suggest that this flooding would enter into the site.
- 4.25 On the above basis, the risk of surface water flooding is considered to be low.
- 4.26 Flooding from Groundwater - Flooding from groundwater can happen when groundwater levels are high. This may be due to rainfall in the groundwater source area, but can also happen on floodplains if river levels are held above the level of the flood plain by embankments.
- 4.27 The West Lancashire Borough Council Strategic Flood Risk Assessment (SFRA) Level 1 (December 2019) notes that *"the majority of West Lancashire is at low risk from groundwater flooding. However, areas of Skelmersdale [...] are at higher risk"*. Appendix 5 of the Level 1 SFRA includes broad-scale mapping giving an indication of susceptibility to groundwater emergence based on general prevailing geological factors. This mapping indicates that the site lies within a 1km² grid squares where between 25% and 50% of the area may be susceptible to

groundwater emergence. i.e. there is a relatively low geological potential for groundwater emergence in that 1km² area.

- 4.28 More detailed mapping included in the ‘Spatial Area Summary’ of the West Lancashire Borough Council Level 2 SFRA (May 2020) also indicates areas of potential groundwater flood risk based on different criteria. Although some parts of Skelmersdale are identified as being at significant risk, the majority of the site and immediate surrounding area are not shown to be in an area with any specific risk of groundwater flooding. However, there is a narrow band of elevated risk running north to south through the site, where it is suggested that groundwater levels may be near to the surface (between 0.025m and 0.5m deep). From the scale of the mapping it is not clear exactly where this band is located within the site.
- 4.29 ARP has prepared a Stage 1 Desk Study Report (reference NW134/01/2r1) which included British Geological Survey data on susceptibility to flooding from groundwater as part of an ‘Envirocheck’ report on the site. This mapping indicates the general area has the “potential for groundwater flooding to occur at surface”. However, it must be noted this mapping is indicative only based on general geological conditions, and is not an absolute indication of risk, only an indication that further consideration is required.
- 4.30 ARP has also prepared a Stage 2 Geo-Environmental Report (reference NW134/01/2r2). This report is informed by intrusive ground investigation undertaken by ARP in 2020. The investigation comprised fifteen machine excavated trial pits and eleven boreholes to depths of between 2.43m and 5.45m, and one hand-dug trial pit to a depth of 0.3m. Slight groundwater seepages were recorded at 4.5m depth in one trial pit, located towards the southwest of the site. Groundwater was not encountered in any of the other investigation positions.
- 4.31 As noted in Section 2, the two watercourses on site were observed to have very little baseflow during two site visits undertaken by ARP in January 2020 and September 2021, and when the topographical survey and update survey were undertaken in August 2020 and January 2022. A steady flow was observed in the watercourses during the site visit undertaken in November 2021, but this was on a day of very heavy and prolonged rainfall, which was most likely the

source of the flow. Based on the depth of the channels, if local groundwater levels were elevated, it would be expected that the watercourses would have a base flow over at least some part of the year.

- 4.32 On balance, given that the physical evidence does not suggest a high groundwater level, the risk of groundwater flooding at the site is considered to be low.
- 4.33 Flooding from Drains and Sewers - There are no existing public sewers within the site itself. Existing public sewers in the residential roads surrounding the site are separate systems of foul and surface water, and based on the approximate age of the housing stock they can be assumed to be of relatively recent construction. United Utilities is responsible for the maintenance of the existing public sewer systems.
- 4.34 The topography of the surrounding roads is such that, in the event flooding did occur from these existing sewers, it is unlikely to enter into the proposed development. If flooding in roads to the south and west was significant, it could potentially enter into the site boundary, but it would be intercepted by the existing watercourses.
- 4.35 The proposed development will not lead to any increase in surface water flows into the public sewers, and United Utilities has confirmed that the foul public sewers have the capacity to receive foul flows from the proposed development.
- 4.36 Based on the above the risk of flooding from existing drains and sewers is considered to be low.
- 4.37 Flooding from Reservoirs, Canals or Artificial Sources - The Environment Agency produce maps which show the expected inundation area should a reservoir fail and release its capacity, but the site is not shown to be in an area at risk of flooding from this source.
- 4.38 There are no further canals or artificial sources located near to the site, therefore the risk of flooding from this source is deemed to be low.

Climate Change

- 4.39 NPPF requires that the projected impacts of climate change are taken into account over the lifetime of a development. The NPPF and PPG have indicated that the Global sea level will continue to rise, depending on greenhouse gas emissions, and the sensitivity of the climate system and there will be an increase in river flows and rainfall across the country. United Kingdom climate change guidance was revised in February 2016; Recommended allowances for assessment are set out in Environment Agency publication 'Flood risk assessments: climate change allowances' (February 2016; updated October 2021).
- 4.40 Impact on River Flooding - With regards to peak river flows, a regionalised approach has now been adopted to climate change impacts based upon the management catchment of the proposed development site, the flood risk vulnerability of the proposed development and the present-day Flood Zone classification. For residential development, climate change impacts over a design life of 100 years should be considered.
- 4.41 The site is situated within the Douglas Management Catchment of the North West river basin district. Although an applicable allowance for sites in Flood Zone 1 is not specifically given by the current guidance, as a precautionary approach it is reasonable that the impacts of the "central" climate change scenario (50th percentile of potential scenarios), should be considered with respect to the proposed development. In this management catchment, the 'central' climate change scenario is predicted to lead to an increase in peak river flows of 35% by 2115 (which is as far as the data extends).
- 4.42 An increase in flow in the watercourses which flow through the site is most likely to result in surcharging in the south of the site, where the two watercourses confluence and enter a culvert, in the event that the increased flow overwhelms the capacity of the downstream culvert. However, as discussed above, if this were to occur the flood level would not be expected to exceed approximately 95.0mAOD before the flood waters overtopped into Mercury Way. On this basis the risk of fluvial flooding in the future is considered to be limited to flooding up to a level

of approximately 95.0mAOD, and further measures to mitigate this residual risk are set out below.

- 4.43 Impact on Rainfall Intensity - In accordance with the current climate change guidance, the published figures show that for an expected development life of greater than 50 years, the anticipated increase in rainfall intensity could range from 20% to 40%, subject to the location within the country.
- 4.44 As required by the LLFA, the outline surface water drainage strategy (discussed further below) has been defined with respect to a precautionary 40% allowance.

Proposed Watercourse Crossing

- 4.45 It is proposed that a new adoptable road crossing will be constructed over the westernmost watercourse channel to facilitate access to the eastern part of the site.
- 4.46 Construction of any new crossing introduces a potential risk of upstream flooding due to afflux should the flow in the watercourse be impeded by the crossing and cause the upstream water level to increase. Therefore, design of the crossing will need to be undertaken with reference to the expected flood flows in the watercourse.
- 4.47 The crossing design is likely to be based around a culvert structure to maintain flow within the watercourse. Based on the likely flows in the watercourse at this point (as discussed above), and considering that the culvert size should be large enough to mitigate the risk of regular blockage, it is considered that a culvert of around 600mm diameter (or box-section equivalent) would be appropriate in this location.
- 4.48 Further consultation with the Lead Local Flood Authority should be undertaken at the time of design, to confirm any specific design requirements, including agreement of the culvert size. In addition to the requirement to pass flows unrestricted, further design considerations may include: ecological considerations such as provision of wildlife ledges and provision of a

naturalised bed; access requirements for maintenance; and requirements such as the provision of trash screens to reduce blockage risk.

- 4.49 As the crossing is required to carry an adoptable highway, the design should also be discussed and agreed with the Highway Authority which may have further design requirements which will need to be taken into account.
- 4.50 Assuming implementation of the above, and subject to further approval of the design by the Highway Authority, and the LLFA through the Land Drainage Consent process (see below), the proposed crossing would not be expected to have a detrimental impact on flood risk, and the risk of afflux flooding from proposed watercourse crossing is considered to be low.

Proposed Surface Water Drainage

- 4.51 It is a requirement of the NPPF to ensure that surface water run-off from any proposed development has negligible consequence on downstream areas either in terms of impact upon downstream sewer capacity or flood risk impacts due to discharge to a watercourse. The existing and proposed surface water run-off is summarised below.
- 4.52 Further details of the proposed surface water drainage strategy are set out in the ARP Foul and Surface Water Drainage Strategy report (reference NW134/01/2r4) and the preliminary civil engineering design drawings and calculations which are scheduled in Section 6.2 of the Drainage Strategy report.
- 4.53 Existing Surface Water Run-off - As set out above, survey has determined that surface water from the positively drained area around the existing building and playground is drained to a combined drainage system which discharges from the east of the site, to a public sewer within the rear garden of a neighbouring property. There is no positive drainage across the majority of the site - Some runoff may infiltrate to a degree, but it is likely that the majority simply run towards the lowest areas of the site and into the watercourses within the east of the site.

- 4.54 Using the HR Wallingford UKSuDS online tool, and applying the IoH124 methodology (Interim Code of Practice for SuDS method for site area less than 50ha), the 1 in 1 year greenfield run-off rate for the proposed development site has been assessed to be 6.1l/s/ha. The calculation is presented in Appendix G.
- 4.55 Proposed Surface Water Runoff Destination - The proposed means of managing surface water runoff from the proposed development has been considered with respect to the hierarchy set out in Building Regulations Part H (2010).
- 4.56 The potential for use of infiltration has been considered by ARP as set out in the Foul and Surface Water Drainage Strategy report (reference NW134/01/2/r4), based on the findings of the ARP Stage 2 Geo-Environmental Report (reference NW134/01/2r2). It has been concluded that, accounting for the site's underlying geology, presence of underlying historic mine workings, hydrogeology and the topography of the site and surrounding area, the use of soakaway drainage is not a viable drainage option for this site.
- 4.57 As infiltration is unfeasible, discharge to the watercourse which passes through the site is the most sustainable option for drainage of the site. Preliminary design has confirmed that an outfall to the watercourse can be achieved by gravity. Details of the outfall will be subject to further design and agreement with the LLFA through the Land Drainage Consent process (discussed further below).
- 4.58 Proposed Surface Water Drainage – It is proposed that the surface water discharge to the watercourse will be attenuated to a maximum rate of 10.4l/s, based on the 1 in 1 year greenfield equivalent rate from the developable area of the site, such that the discharge of surface water does not cause a determinantal increase in the flow in the watercourse or an increase in flood risk.
- 4.59 The proposed surface water system will be designed to accommodate a 1 in 30 year storm event without flooding, and the 1 in 100 year plus 40% climate change allowance event without

causing flooding of property or third-party land. An appropriate volume of storage will be accommodated on site by means of an attenuation tank.

4.60 The strategy above is subject to agreement with the Regulatory Authorities.

4.61 Residual Risk - As with any new drainage system, it is possible that flooding may occur from the lowest cover level of manholes or gullies in the event that the system becomes blocked or is overwhelmed. The potential for any drainage outfall to be surcharged will also need to be considered as per the joint probability assessment requirements of the Lead Local Flood Authority. The above will need to be considered as part of any proposed development, with potential exceedance flows directed away from property to be contained in less vulnerable areas of the site as far as practicable.

Flood Mitigation

4.62 The site falls within Flood Zone 1, and the risk from the majority of sources has been assessed as being low. However, there is a residual risk of flooding from the watercourses within the site, and there is a need to consider the topography of the site in terms of the residual risk of surface water flooding due to blockage or exceedance of drainage system capacity. The following precautionary mitigation measures are, therefore, recommended:

4.62.1 Finished floor levels FFLs shall be set at or above 95.6mAOD i.e. 600mm above the expected maximum fluvial flood level of 95.0mAOD, as a precautionary approach to managing the residual risk of fluvial flooding.

4.62.2 The finished floor levels of buildings shall be raised above general external levels by a minimum of 150mm.

4.62.3 The proposed ground floors shall comprise solid concrete slabs or beam and block floors with screed construction.

- 4.62.4 Incoming electricity supplies shall be raised above ground floor level (after the meter) and ground floor electric sockets shall be served by loops from upper levels.
- 4.62.5 A sustainable surface water drainage system shall be provided to manage surface water run-off from the site itself up to the 1 in 100 year plus climate change event.
- 4.62.6 External levels should be designed with falls to direct overland flows away from the building entrances where possible, so that any flooding remains in less vulnerable areas such as landscaped areas, car parks, or roads, where the consequences of surface water flooding would be less significant. Where falls towards buildings are unavoidable, additional cut-off drainage and gullies/channel drains should be provided to prevent water entering buildings during extreme events.
- 4.62.7 A minimum 5m stand-off from the watercourses within to the site (measured from the top of the watercourse bank) shall be maintained, with suitable access for future watercourse maintenance.
- 4.62.8 Detailed design of the proposed crossing of the watercourse should be undertaken with reference to the requirements of the LLFA and the Highway Authority.
- 4.63 Any mitigation measures, including drainage systems, would require suitable maintenance systems to be implemented, so that the design standard is maintained over the lifetime of the development.

Land Drainage Consent

- 4.64 Land Drainage Consents (from the Lead Local Flood Authority) will be required for the construction of any new surface water outfall to the watercourses on site, and for the new watercourse crossing.

- 4.65 A detailed design for these structures, along with construction method statements and other supporting evidence, will need to be submitted to the LLFA for approval in due course, following further design development.
- 4.66 It should be noted that the granting of planning permission does not guarantee the granting of Land Drainage Consents. However, the proposed works would not be expected to have any detrimental impact upon the watercourses, therefore it is not envisaged that there will be any barrier to securing consents.

5.0 SUMMARY

- 5.1 The site is located in Flood Zone 1, indicating a 'low' risk of flooding from rivers and the sea. On this basis, application of the NPPF Sequential Test and Exception Test is not required.
- 5.2 The risk from the majority of sources has been assessed as being low. However, there is a residual risk of flooding from the watercourses within the site, and there is a need to consider the topography of the site in terms of the residual risk of surface water flooding due to blockage or exceedance of drainage system capacity. The following precautionary mitigation measures are, therefore, recommended:
- 5.2.1 Finished floor levels FFLs shall be set at or above 95.6mAOD i.e. 600mm above the expected maximum fluvial flood level of 95.0mAOD, as a precautionary approach to managing the residual risk of fluvial flooding.
 - 5.2.2 The finished floor levels of buildings shall be raised above general external levels by a minimum of 150mm.
 - 5.2.3 The proposed ground floors shall comprise solid concrete slabs or beam and block floors with screed construction.
 - 5.2.4 Incoming electricity supplies shall be raised above ground floor level (after the meter) and ground floor electric sockets shall be served by loops from upper levels.
 - 5.2.5 A sustainable surface water drainage system shall be provided to manage surface water run-off from the site itself up to the 1 in 100 year plus climate change event.
 - 5.2.6 External levels should be designed with falls to direct overland flows away from the building entrances where possible, so that any flooding remains in less vulnerable areas such as landscaped areas, car parks, or roads, where the consequences of surface water flooding would be less significant. Where falls towards buildings are

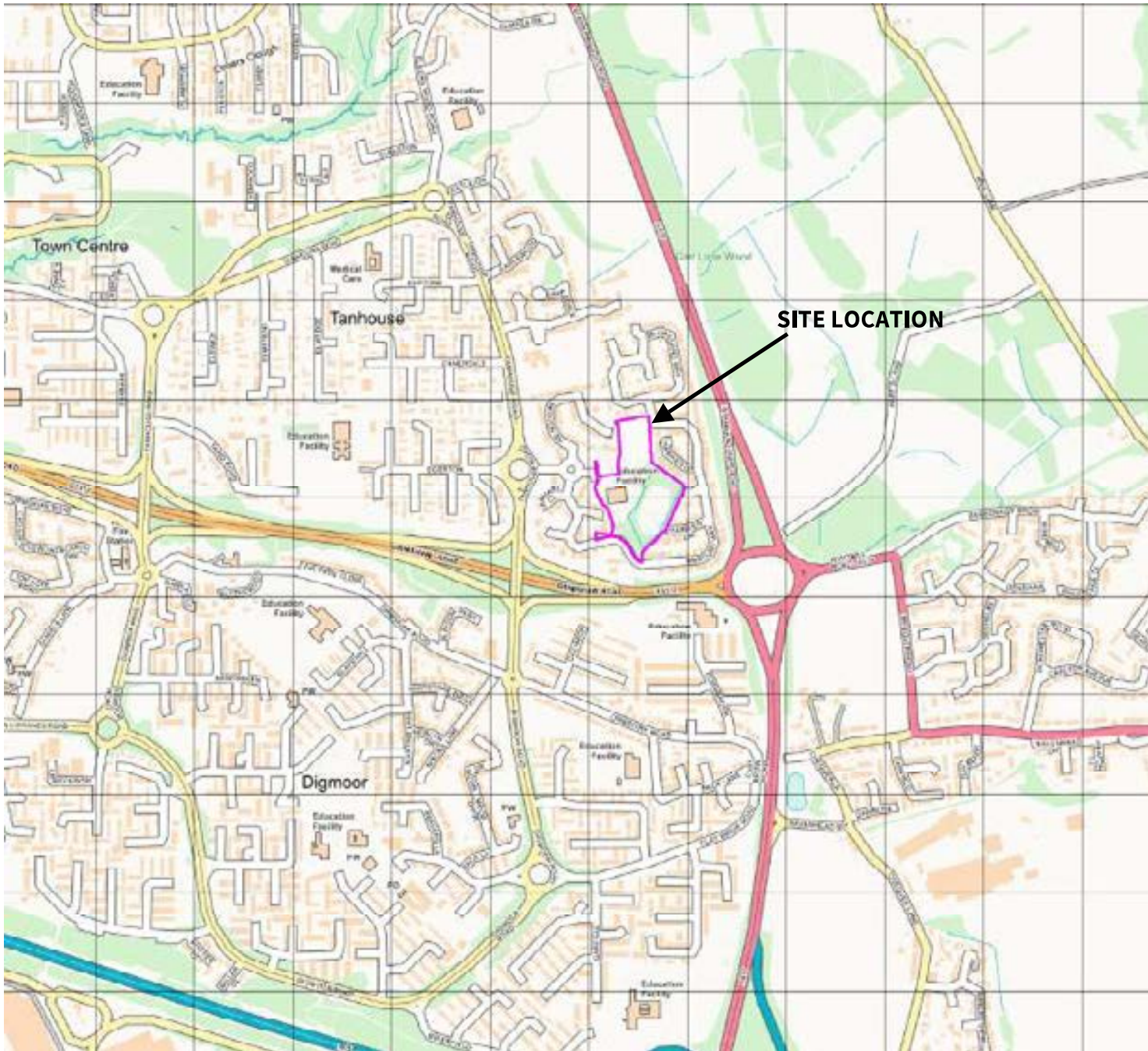
unavoidable, additional cut-off drainage and gullies/channel drains should be provided to prevent water entering buildings during extreme events.

- 5.2.7 A minimum 5m stand-off from the watercourses within to the site (measured from the top of the watercourse bank) shall be maintained, with suitable access for future watercourse maintenance.
- 5.2.8 Detailed design of the proposed crossing of the watercourse should be undertaken with reference to the requirements of the LLFA and the Highway Authority.
- 5.3 The principles of a sustainable surface water management strategy for the proposed development are outlined within the report. Surface water will be discharge to the watercourse which flows through the site before flowing south. The proposed maximum surface water discharge rate will be 10.4l/s, based on the 1 in 1 year greenfield equivalent runoff rate from the developable area of the site, such that the discharge will not be expected to lead to any increase in flood risk elsewhere.
- 5.4 Surface water storage will be provided within a tank, to manage surface water up to the 1 in 100 year rainfall event, including an additional 40% allowance for the projected impacts of climate change. An allowance of 10% on impermeable areas has also been made to account for the potential impacts of 'urban creep'.
- 5.5 Further details of the proposed surface water drainage strategy are set out in the ARP Foul and Surface Water Drainage Strategy report (reference NW134/01/2r4) and the preliminary civil engineering design drawings and calculations which are scheduled in Section 6.2 of the Drainage Strategy report.
- 5.6 Any mitigation measures, including drainage systems, will require suitable maintenance systems to be implemented, so that the design standard is maintained over the lifetime of the development.

- 5.7 A Land Drainage Consent (under the Land Drainage Act 1991) will be required from the LLFA for the construction of any new outfall to the watercourse within the site, and for the construction of the proposed crossing of the watercourse.
- 5.8 The findings of this report are subject to the approval of the Regulatory Authorities.
- 5.9 Subject to compliance with the above, the proposed development can satisfy the requirements of the National Planning Policy Framework and the Planning Practice Guidance in relation to flood risk.

APPENDIX A

SITE LOCATION PLAN



0m 400m

Approximate Scale

ARP
 ARP NORTH WEST LTD
 CHARTERED CONSULTING ENGINEERS
 2nd Floor · 3 Hardman Square · Manchester M3 3EB
 Telephone: 0113 245 8498 · E-Mail: manchester@arnorthwest.co.uk

Project HOPE HIGH SCHOOL SKELMERSDALE	
Client TAWD VALLEY DEVELOPMENTS LIMITED	
Title SITE LOCATION PLAN	
Date JANUARY 2020	
Drawn DMB	Scale AS SHOWN
Job No. NW134/01/02	



0m 100m
Approximate Scale

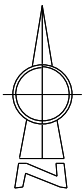


2nd Floor · 3 Hardman Square · Manchester M3 3EB
Telephone: 0113 245 8498 · E-Mail: manchester@arnorthwest.co.uk

Project HOPE HIGH SCHOOL SKELMERSDALE	
Client TAWD VALLEY DEVELOPMENTS LIMITED	
Title AERIAL PHOTOGRAPH	
Date JANUARY 2020	
Drawn DMB	Scale AS SHOWN
Job No. NW134/01/02	

APPENDIX B

SITE TOPOGRAPHY



N O T E S

ATTENTION IS DRAWN TO THE REQUIREMENTS OF THE CONSTRUCTION DESIGN AND MANAGEMENT REGULATIONS 2015 AND THE DUTIES AND RESPONSIBILITIES CONTAINED THEREIN

1. Do not scale from this drawing.
2. Topographical survey undertaken by PD Site Surveys on behalf of ARP (North West) Ltd. Date of original survey 05/08/2020. Supplementary survey undertaken 22/11/2021.
3. Survey based on OS Grid and Level as surveyed with GPS OS GB36-15. Grid set at 25m - contours set at 500mm.
4. This survey shows physical site boundaries only. Confirmation of legal ownership boundaries should be obtained by reference to the H.M. Land Registry Title Plan. Every effort is made to identify all visible above ground features. However, it should be borne in mind that there may be items obscured at the time of survey.
5. The contractor is to check and verify all building and site dimensions, levels and sewer invert levels at connection points before work starts. Should there be any conflict between the details indicated on this drawing and those indicated on other drawings, the engineer should be informed prior to construction on site.

SYMBOL LEGEND

- | | |
|-----------------------------|---------------------------|
| ⊕ Borehole | ✕ Pylon |
| ● Benchmark | ● Rodding Eye |
| ● Bollard | □ BT Exchange Box |
| ● Bus Stop | ○ Sign Post |
| □ Electric Box | ○ Spot Height |
| ⊕ Electricity Pole | ⊕ Eave Level |
| ● Fence Post | ⊕ Ridge Level |
| ● Gas Stop Valve | ✱ Stile |
| ● Gate Post | ▲ Survey Station |
| ● Gas Marker | ▼ Traffic Camera |
| □ Gully Drain | □ Telephone Box |
| ⊕ Electric Inspection Cover | ○ Trial Pit |
| ⊕ Kerb Outlet | ○ Telegraph Pole |
| □ Litter Bin | ○ Deciduous Tree |
| □ BT Inspection chamber | ○ Road Sign |
| ○ Lamp Post | — GATE |
| ⊕ Manhole Point | □ Fire Hydrant |
| △ MHT Triangular | □ Water Meter |
| □ MHT Square | ○ Letter Box |
| ○ MHT Round | □ CATV Inspection Chamber |
| ● Marker Post | ⊕ Water Stop Valve |

ABBREVIATIONS

- | | |
|-------|------------------------|
| LP. | Lamp Post |
| TP. | Telegraph Pole |
| EP. | Electricity Pole |
| SP. | Sign Post |
| TS. | Tarffic Sign |
| BS. | Bus Stop |
| GM. | Gas Marker |
| MH. | Manhole |
| GU. | Gully |
| FH. | Fire Hydrant |
| CATV. | Cable Television Point |
| WM. | Water Meter |
| WSV. | Water Stop Valve |
| GSV. | Gas Stop Valve |
| RE. | Rodding Eye |

A	JC	14.01.22	Amended to new survey with additional areas	MGW	AR
/	MGW	14.08.20	For information	MGW	AR

Rev	By	Date	Revision	Chk	Apvd
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TITLE
TOPOGRAPHICAL SURVEY
SHEET 1 OF 2

PROJECT
FORMER HOPE HIGH SCHOOL

CLIENT
TAWD VALLEY DEVELOPMENTS

DRAWING STATUS
FOR INFORMATION

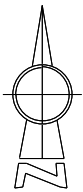
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1:500 @ A1	AUG 20	Chk.	MGW

Orig. No.	Rev
NW134/01/2/00.1	A

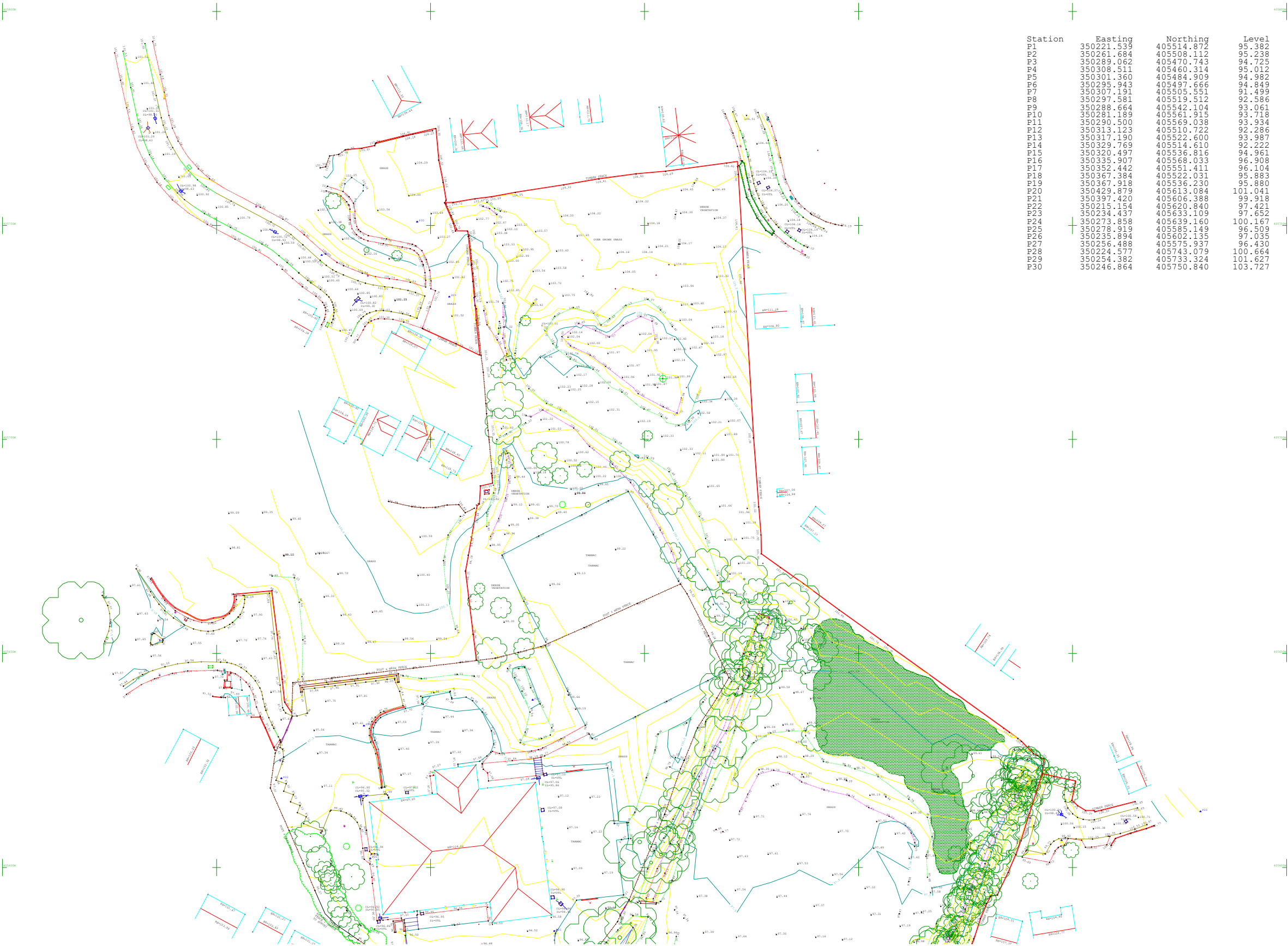
Station	Easting	Northing	Level
P1	350221.539	405514.872	95.382
P2	350261.684	405508.112	95.238
P3	350289.062	405470.743	94.725
P4	350308.511	405460.314	95.012
P5	350301.360	405484.909	94.982
P6	350295.943	405497.666	94.849
P7	350307.191	405505.551	91.499
P8	350297.581	405519.512	92.586
P9	350288.664	405542.104	93.061
P10	350281.189	405561.915	93.718
P11	350290.500	405569.038	93.934
P12	350313.123	405510.722	92.286
P13	350317.190	405522.600	93.987
P14	350329.769	405514.610	92.222
P15	350320.497	405536.816	94.961
P16	350335.907	405568.033	96.908
P17	350352.442	405551.411	96.104
P18	350367.384	405522.031	95.883
P19	350367.918	405536.230	95.880
P20	350429.879	405613.084	101.041
P21	350397.420	405606.388	99.918
P22	350215.154	405620.840	97.421
P23	350234.437	405633.109	97.652
P24	350273.858	405639.160	100.167
P25	350278.919	405585.149	96.509
P26	350235.894	405602.135	97.035
P27	350256.488	405575.937	96.430
P28	350224.577	405743.079	100.664
P29	350254.382	405733.324	101.627
P30	350246.864	405750.840	103.727

LINETYPE LEGEND

- | | | |
|----------------|---------------------|---------------------|
| Bridge | Path | Flagstone Paving |
| Bottom of Bank | Pipe Line | Stone |
| Building | Foul Sewer | Hedgeline |
| Canopy Edge | Surface Water Sewer | Grass Edge |
| Concrete Edge | Chanel Line | Vegetation Line |
| Ditch | Road Centre Line | Post and Rail Fence |
| Drop Kerb | Tarmac Edge | Post and Wire Fence |
| Overhead Cable | Top of Bank | Contour Normal |
| Grid | Track | Contour Prominent |
| Tackile Paver | Wall | |
| Kerb | Verge | |



LINETYPE LEGEND					
	Bridge		Path		Flagstone Paving
	Bottom of Bank		Pipe Line		Stone
	Building		Foul Sewer		Hedgeline
	Canopy Edge		Surface Water Sewer		Grass Edge
	Concrete Edge		Chanel Line		Vegetation Line
	Ditch		Road Centre Line		Post and Rail Fence
	Drop Kerb		Tarmac Edge		Post and Wire Fence
	Overhead Cable		Top of Bank		Contour Normal
	Grid		Track		Contour Prominent
	Tacktile Paver		Wall		
	Kerb		Verge		



Station	Easting	Northing	Level
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P15	350320.497	405536.816	94.961
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P17	350352.442	405551.411	96.104
P18	350367.384	405522.031	95.883
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P28	350224.577	405743.079	100.664
P29	350254.382	405733.324	101.627
P30	350246.864	405750.840	103.727

SYMBOL LEGEND

	Borehole		Pylon
	Benchmark		Rodding Eye
	Bollard		BT Exchange Box
	Bus Stop		Sign Post
	Electric Box		Spot Height
	Electricity Pole		Eave Level
	Fence Post		Ridge Level
	Gas Stop Valve		Stile
	Gate Post		Survey Station
	Gas Marker		Traffic Camera
	Gully Drain		Telephone Box
	Electric Inspection Cover		Trial Pit
	Kerb Outlet		Telegraph Pole
	Litter Bin		Deciduous Tree
	BT Inspection chamber		Road Sign
	Lamp Post		GATE
	Manhole Point		Fire Hydrant
	MH Triangular		Water Meter
	MH Square		Letter Box
	MH Round		CATV Inspection Chamber
	Marker Post		Water Stop Valve

ABBREVIATIONS

LP.	Lamp Post
TP.	Telegraph Pole
EP.	Electricity Pole
SP.	Sign Post
TS.	Traffic Sign
BS.	Bus Stop
GM.	Gas Marker
MH.	Manhole
GU.	Gully
FH.	Fire Hydrant
CATV.	Cable Television Point
WM.	Water Meter
WSV.	Water Stop Valve
GSV.	Gas Stop Valve
RE.	Rodding Eye

A	JC	14.01.22	Amended to new survey with additional areas	MGW	AR
/	MGW	14.08.20	For information	MGW	AR

Rev	By	Date	Revision	Chk	Apvd
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TITLE	
TOPOGRAPHICAL SURVEY SHEET 2 OF 2	
PROJECT	
FORMER HOPE HIGH SCHOOL	
CLIENT	
TAWD VALLEY DEVELOPMENTS	
DRAWING STATUS	
FOR INFORMATION	
Scale	Date
1:500 @ A1	AUG 20
Drawn	MGW
Chk.	MGW
Orig. No.	Rev
NW134/01/2/00.2	A

APPENDIX C

DRAINAGE SURVEY



CCTV SURVEY INSPECTION REPORT

Client:	ARP Geotechnical Ltd
Location:	Hope High School, Holland Park, Skelmersdale
Date:	17.11.2021



Industrial Water Jetting Systems Ltd

Dickens House, Old Stowmarket Road, Woolpit, Bury St. Edmunds IP22 9QS

Tel: 01359 242606 Fax: 01359 242 621

Web: www.iwjs.co.uk Email: surveys@iwjs.co.uk



Project

Project Name: 2021_11_24 ARP Hope High School M Sweeney

Project Date: 24/11/2021

Inspection Standard: MSCC5 Sewers & Drainage GB (SRM5 Scoring)

Table of Contents

Project Name	Project Number	Project Date
2021_11_24 ARP Hope High School M Sweeney		24/11/2021

Project Information	P-1
Scoring Summary	P-2
Section Item 1: OUTLET D > UNK (OUTLET DX)	1
Section Item 2: UNKNOWN > OUTLET A (UNKNOWNX)	3
Section Item 3: UNKNOWN > OUTLET B (UNKNOWNX)	5
Section Item 4: UNKNOWN > OUTLET C (UNKNOWNX)	7
Section Item 5: SW02 > SW01 (SW02X)	8

Project Information

Project Name	Project Number	Project Date
2021_11_24 ARP Hope High School M Sweeney		24/11/2021

Client

Company: ARP Geotechnical Ltd
Street: Northwest House, 5/6 Northwest Business Park
Town or City: Servia Hill, Leeds
County: West Yorkshire
Post Code: LS6 2QH

Site

Company: ARP Geotechnical Ltd
Street: Hope High School, Holland Park
Town or City: Skelmersdale
County: Lancashire
Post Code: WN8 6BJ

Contractor

Company: IWJS
Contact: Sharna Donovan
Street: 10 Huntsman Drive
Town or City: Irlam
County: Greater Manchester
Post Code: M44 5EG
Phone: 0161 776 0499
Mobile: 07866 072580
Email: Sharna.Donovan@iwjs.co.uk

Scoring Summary

Project Name	Project Number	Project Date
2021_11_24 ARP Hope High School M Sweeney		24/11/2021

Structural Defects

- Grade 3: Best practice suggests consideration should be given to repairs in the medium term.
- Grade 4: Best practice suggests consideration should be given to repairs to avoid a potential collapse.
- Grade 5: Best practice suggests that this pipe is at risk of collapse at any time. Urgent consideration should be given to repairs to avoid total failure.

Section	PLR	Grade	Description
3	UNKNOWNX	3	Cracks, multiple at joint from 7 o'clock to 5 o'clock

Service / Operational Condition

- Grade 3: Best practice suggests consideration should be given to maintenance activities in the medium term.
- Grade 4: Best practice suggests consideration should be given to maintenance activity to avoid potential blockages.
- Grade 5: Best practice suggests that this pipe is at a high risk of backing up or causing flooding.

Section	PLR	Grade	Description
3	UNKNOWNX	4	Settled deposits, fine, 40% cross-sectional area loss
4	UNKNOWNX	4	Settled deposits, fine, 20% cross-sectional area loss

Abandoned Surveys

Section	PLR	Description
1	OUTLET DX	Survey abandoned
2	UNKNOWNX	Survey abandoned
3	UNKNOWNX	Survey abandoned
4	UNKNOWNX	Survey abandoned
5	SW02X	Survey abandoned

Information

These scoring summaries are based on the SRM grading from the WRc.

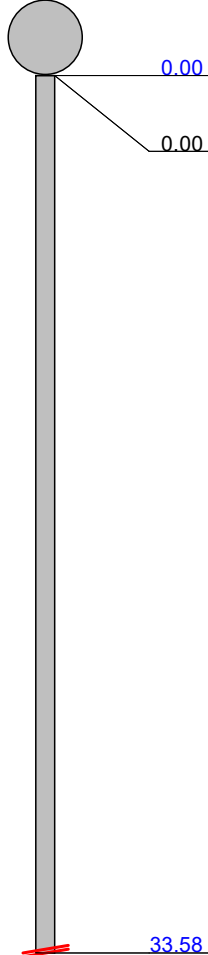
Section Inspection - 24/11/2021 - OUTLET DX



Item No. 1	Insp. No. 1	Date 24/11/21	Time 12:02	Client's Job Ref Not Specified	Weather No Rain Or Snow	Pre Cleaned No	PLR OUTLET DX
Operator M Sweeney		Vehicle FL65 WZD		Camera Small Crawler	Preset Length Not Specified	Legal Status Watercourse	Alternative ID Not Specified

Town or Village:	Skemersdale	Inspection Direction:	Downstream	Upstream Node:	OUTLET D
Road:	Hope High School	Inspected Length:	33.58 m	Upstream Pipe Depth:	2.150 m
Location:	Fields, farmland etc	Total Length:	33.58 m	Downstream Node:	UNK
Surface Type:		Joint Length:		Downstream Pipe Depth:	2.180 m
Use:	Surface water	Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer	Dia/Height:	750 mm		
Flow Control:	No flow control	Material:	Concrete		
Year Constructed:	Not Specified	Lining Type:	No Lining		
Inspection Purpose:	Sample condition survey	Lining Material:	No Lining		

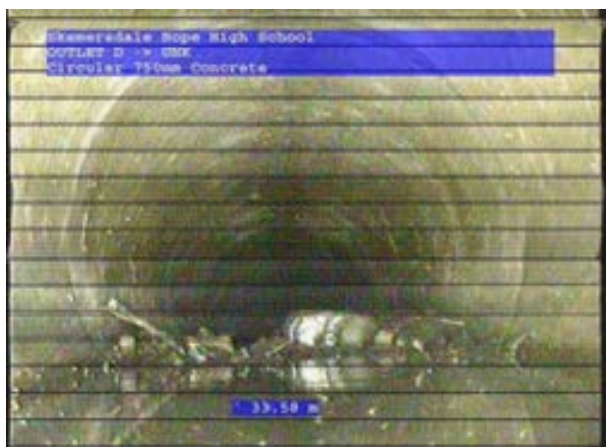
Comments:
Recommendations:

Scale: 1:290	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: 2.15 m						
OUTLET D						
						
	0.00	WL	Water level, 5% of the vertical dimension	00:00:00	OUTLET DX_8e641eb6-820b-	
	0.00	OF	Start node type, outfall, reference number: OUTLET D	00:00:00	OUTLET DX_689b5e4d-7a8f-4	
	33.58	SA	Survey abandoned: Cleaning req	00:04:23		

Construction Features					Miscellaneous Features				
Structural Defects					Service & Operational Observations				
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
0	0.0	0.0	0.0	1.0	0	0.0	0.0	0.0	1.0

Section Pictures - 24/11/2021 - OUTLET DX

Item No.	Inspection Direction	PLR	Client's Job Ref	Contractor's Job Ref
1	Downstream	OUTLET DX		



OUTLET

DX_8e641eb6-820b-4843-93d4-61bf13e2ea86_20211124_12
1320_042.jpg, 00:00:00, 0.00 m
Water level, 5% of the vertical dimension



OUTLET

DX_689b5e4d-7a8f-4e8b-853a-ad2c0e5dc76b_20211124_12
1303_590.jpg, 00:00:00, 0.00 m
Start node type, outfall, reference number: OUTLET D

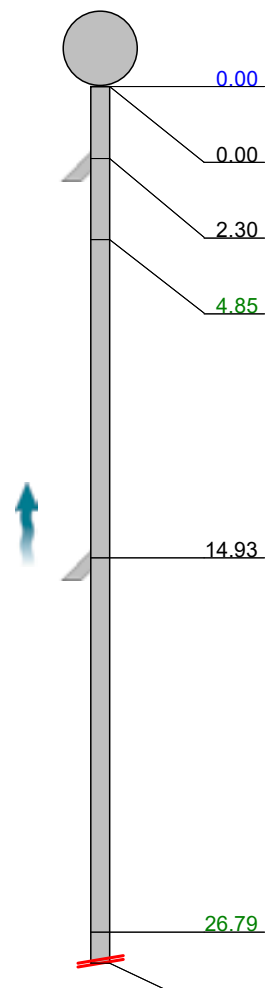
Section Inspection - 25/11/2021 - UNKNOWNX



Item No. 2	Insp. No. 2	Date 25/11/21	Time 9:36	Client's Job Ref Not Specified	Weather No Rain Or Snow	Pre Cleaned No	PLR UNKNOWNX
Operator M Sweeney		Vehicle FL65 WZD		Camera Push Rod	Preset Length Not Specified	Legal Status Watercourse	Alternative ID Not Specified

Town or Village:	Skemersdale	Inspection Direction:	Upstream	Upstream Node:	UNKNOWN
Road:	Hope High School	Inspected Length:	27.77 m	Upstream Pipe Depth:	
Location:	Fields, farmland etc	Total Length:	27.77 m	Downstream Node:	OUTLET A
Surface Type:		Joint Length:		Downstream Pipe Depth:	1.890 m
Use:	Surface water	Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer	Dia/Height:	150 mm		
Flow Control:	No flow control	Material:	Vitrified clay		
Year Constructed:	Not Specified	Lining Type:	No Lining		
Inspection Purpose:	Sample condition survey	Lining Material:	No Lining		

Comments:
Recommendations:

Scale: 1:240	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: 1.89 m OUTLET A						
						
	0.00	WL	Water level, 5% of the vertical dimension			
	0.00	OF	Start node type, outfall, reference number: OUTLET A	00:00:00		
	2.30	JN	Junction at 2 o'clock, diameter: 100mm	00:00:15		
	4.85	LL	Line deviates left	00:00:29		
	14.93	JN	Junction at 2 o'clock, diameter: 100mm	00:01:23		
	26.79	DER	Settled deposits, coarse, 15% cross-sectional area loss: Brick	00:02:45	UNKNOWNX_f373b58d-0d7d-	3
	27.77	SA	Survey abandoned: Due to brick in line. Clean required	00:00:00		

Construction Features					Miscellaneous Features				
Structural Defects					Service & Operational Observations				
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
0	0.0	0.0	0.0	1.0	1	2.0	0.1	2.0	3.0

Section Pictures - 25/11/2021 - UNKNOWNX

Item No.	Inspection Direction	PLR	Client's Job Ref	Contractor's Job Ref
2	Upstream	UNKNOWNX		



UNKNOWNX_f373b58d-0d7d-4065-9837-d61e995b3a87_202
20110_081840_276.jpg, 00:02:45, 26.79 m
Settled deposits, coarse, 15% cross-sectional area loss, Brick

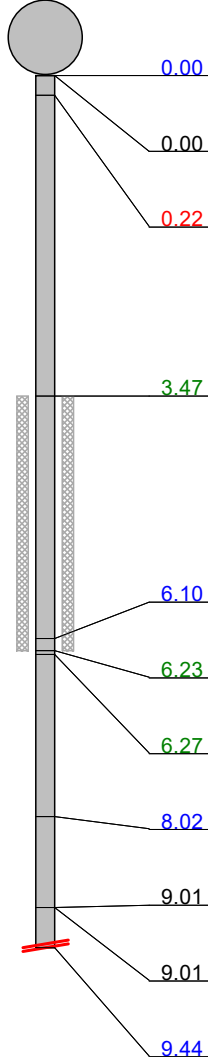
Section Inspection - 25/11/2021 - UNKNOWNX



Item No. 3	Insp. No. 3	Date 25/11/21	Time 9:52	Client's Job Ref Not Specified	Weather No Rain Or Snow	Pre Cleaned No	PLR UNKNOWNX
Operator M Sweeney		Vehicle FL65 WZD		Camera Push Rod	Preset Length Not Specified	Legal Status Watercourse	Alternative ID Not Specified

Town or Village:	Skemersdale	Inspection Direction:	Upstream	Upstream Node:	UNKNOWN
Road:	Hope High School	Inspected Length:	9.44 m	Upstream Pipe Depth:	
Location:	Fields, farmland etc	Total Length:	9.44 m	Downstream Node:	OUTLET B
Surface Type:		Joint Length:		Downstream Pipe Depth:	2.100 m
Use:	Surface water	Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer	Dia/Height:	150 mm		
Flow Control:	No flow control	Material:	Vitrified clay		
Year Constructed:	Not Specified	Lining Type:	No Lining		
Inspection Purpose:	Sample condition survey	Lining Material:	No Lining		

Comments:
Recommendations:

Scale:	1:82	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: 2.10 m							
OUTLET B							
		0.00	WL	Water level, 5% of the vertical dimension			
		0.00	OF	Start node type, outfall, reference number: OUTLET B	00:00:00		
		0.22	CMJ	Cracks, multiple at joint from 7 o'clock to 5 o'clock	00:00:03	UNKNOWNX_e0e87def-a566-4	3 / 2
		3.47	S01	DES Settled deposits, fine, 10% cross-sectional area loss, start	00:00:18		
		6.10	WL	Water level, 20% of the vertical dimension	00:00:33		
		6.23	F01	DES Settled deposits, fine, 10% cross-sectional area loss, finish	00:00:31		3
		6.27	DES	Settled deposits, fine, 40% cross-sectional area loss	00:00:31		4
		8.02	GP	General photograph taken at this point	00:00:42	UNKNOWNX_2fc346b4-a0b5-	
		9.01	SC	Pipe size changes, new size(s), 100mm high	00:00:48		
		9.01	MCPVC	Pipe material changes to polyvinyl chloride at this point	00:00:48		
		9.44	SA	Survey abandoned: Due to size change	00:00:00		

Construction Features					Miscellaneous Features				
Structural Defects					Service & Operational Observations				
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
1	40.0	4.2	40.0	3.0	3	7.0	1.3	12.0	4.0

Section Pictures - 25/11/2021 - UNKNOWNX

Item No.	Inspection Direction	PLR	Client's Job Ref	Contractor's Job Ref
3	Upstream	UNKNOWNX		



UNKNOWNX_e0e87def-a566-4820-bfd2-1157fa7d0d86_2021
1125_095407_250.jpg, 00:00:03, 0.22 m
Cracks, multiple at joint from 7 o'clock to 5 o'clock



UNKNOWNX_2fc346b4-a0b5-4b5d-b981-410b74c8f632_2022
0110_081929_087.jpg, 00:00:42, 8.02 m
General photograph taken at this point

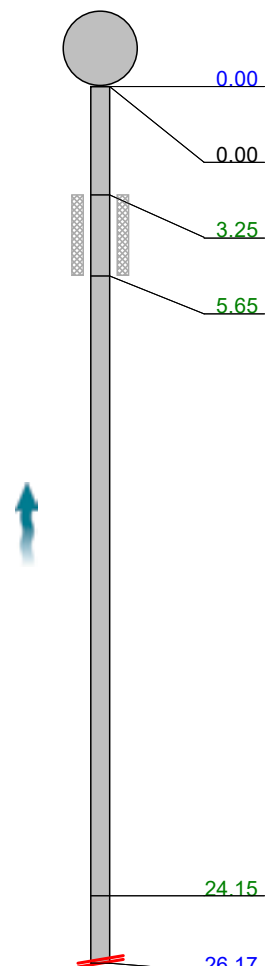
Section Inspection - 25/11/2021 - UNKNOWNX



Item No. 4	Insp. No. 4	Date 25/11/21	Time 10:04	Client's Job Ref Not Specified	Weather No Rain Or Snow	Pre Cleaned No	PLR UNKNOWNX
Operator M Sweeney		Vehicle FL65 WZD		Camera Push Rod	Preset Length Not Specified	Legal Status Watercourse	Alternative ID Not Specified

Town or Village:	Skemersdale	Inspection Direction:	Upstream	Upstream Node:	UNKNOWN
Road:	Hope High School	Inspected Length:	26.17 m	Upstream Pipe Depth:	
Location:	Fields, farmland etc	Total Length:	26.17 m	Downstream Node:	OUTLET C
Surface Type:		Joint Length:		Downstream Pipe Depth:	
Use:	Surface water	Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer	Dia/Height:	300 mm		
Flow Control:	No flow control	Material:	Polyvinyl chloride		
Year Constructed:	Not Specified	Lining Type:	No Lining		
Inspection Purpose:	Sample condition survey	Lining Material:	No Lining		

Comments:
Recommendations:

Scale: 1:226	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: m OUTLET C 						
	0.00	WL	Water level, 5% of the vertical dimension			
	0.00	OF	Start node type, outfall, reference number: OUTLET C	00:00:00		
	3.25	S01	LL	Line deviates left, start		
	5.65	F01	LL	Line deviates left, finish		
	24.15	DES	Settled deposits, fine, 20% cross-sectional area loss			4
	26.17	SA	Survey abandoned: Due to silt Clean required to trace further			

Construction Features					Miscellaneous Features				
Structural Defects					Service & Operational Observations				
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
0	0.0	0.0	0.0	1.0	1	5.0	0.2	5.0	4.0

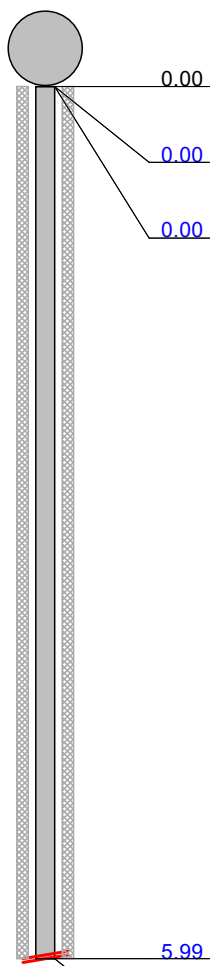
Section Inspection - 11/08/2004 - SW02X



Item No. 5	Insp. No. 1	Date 11/08/04	Time 2:44	Client's Job Ref Not Specified	Weather No Rain Or Snow	Pre Cleaned No	PLR SW02X
Operator M Sweeney		Vehicle FL65 WZD		Camera Small Crawler	Preset Length Not Specified	Legal Status Watercourse	Alternative ID Not Specified

Town or Village:	Skemersdale	Inspection Direction:	Upstream	Upstream Node:	SW02
Road:	Hope High School	Inspected Length:	5.99 m	Upstream Pipe Depth:	
Location:	Fields, farmland etc	Total Length:	5.99 m	Downstream Node:	SW01
Surface Type:		Joint Length:		Downstream Pipe Depth:	
Use:	Surface water	Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer	Dia/Height:	150 mm		
Flow Control:	No flow control	Material:	Vitrified clay		
Year Constructed:	Not Specified	Lining Type:	No Lining		
Inspection Purpose:	Sample condition survey	Lining Material:	No Lining		

Comments:
Recommendations:

Scale:	1:52	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: m SW01 							
		0.00	MH	Start node, manhole, reference: SW01	00:00:00		
		0.00	WL	Water level, 90% of the vertical dimension	00:00:00		
		0.00 S01	CUW	Loss of vision, camera under water, start	00:00:00		
		5.99 F01	CUW	Loss of vision, camera under water, finish	00:01:02		
		5.99	SA	Survey abandoned: Camera under water	00:01:02		

Construction Features					Miscellaneous Features				
Structural Defects					Service & Operational Observations				
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
0	0.0	0.0	0.0	1.0	0	0.0	0.0	0.0	1.0

Grade Defect Report								
Section	Road	m+	Upstream Node	Inspection Direction	Downstream Node	Observation	Structural	Service
1	Hope High School	0	OUTLET D	Downstream	UNK	Water level, 5% of the vertical dimension		
1	Hope High School	0	OUTLET D	Downstream	UNK	Start node type, outfall, reference number: OUTLET D		
1	Hope High School	33.58	OUTLET D	Downstream	UNK	Survey abandoned		
2	Hope High School	0	UNKNOWN	Upstream	OUTLET A	Water level, 5% of the vertical dimension		
2	Hope High School	0	UNKNOWN	Upstream	OUTLET A	Start node type, outfall, reference number: OUTLET A		
2	Hope High School	2.3	UNKNOWN	Upstream	OUTLET A	Junction at 2 o'clock, diameter: 100mm		
2	Hope High School	4.85	UNKNOWN	Upstream	OUTLET A	Line deviates left		
2	Hope High School	14.93	UNKNOWN	Upstream	OUTLET A	Junction at 2 o'clock, diameter: 100mm		
2	Hope High School	26.79	UNKNOWN	Upstream	OUTLET A	Settled deposits, coarse, 15% cross-sectional area loss		3
2	Hope High School	27.77	UNKNOWN	Upstream	OUTLET A	Survey abandoned		
3	Hope High School	0	UNKNOWN	Upstream	OUTLET B	Water level, 5% of the vertical dimension		
3	Hope High School	0	UNKNOWN	Upstream	OUTLET B	Start node type, outfall, reference number: OUTLET B		
3	Hope High School	0.22	UNKNOWN	Upstream	OUTLET B	Cracks, multiple at joint from 7 o'clock to 5 o'clock	3	2
3	Hope High School	3.47	UNKNOWN	Upstream	OUTLET B	Settled deposits, fine, 10% cross-sectional area loss, start		
3	Hope High School	6.1	UNKNOWN	Upstream	OUTLET B	Water level, 20% of the vertical dimension		
3	Hope High School	6.23	UNKNOWN	Upstream	OUTLET B	Settled deposits, fine, 10% cross-sectional area loss, finish		3
3	Hope High School	6.27	UNKNOWN	Upstream	OUTLET B	Settled deposits, fine, 40% cross-sectional area loss		4
3	Hope High School	8.02	UNKNOWN	Upstream	OUTLET B	General photograph taken at this point		
3	Hope High School	9.01	UNKNOWN	Upstream	OUTLET B	Pipe size changes, new size(s), 100mm high		
3	Hope High School	9.01	UNKNOWN	Upstream	OUTLET B	Pipe material changes to polyvinyl chloride at this point		
3	Hope High School	9.44	UNKNOWN	Upstream	OUTLET B	Survey abandoned		
4	Hope High School	0	UNKNOWN	Upstream	OUTLET C	Water level, 5% of the vertical dimension		
4	Hope High School	0	UNKNOWN	Upstream	OUTLET C	Start node type, outfall, reference number: OUTLET C		
4	Hope High School	3.25	UNKNOWN	Upstream	OUTLET C	Line deviates left, start		
4	Hope High School	5.65	UNKNOWN	Upstream	OUTLET C	Line deviates left, finish		
4	Hope High School	24.15	UNKNOWN	Upstream	OUTLET C	Settled deposits, fine, 20% cross-sectional area loss		4
4	Hope High School	26.17	UNKNOWN	Upstream	OUTLET C	Survey abandoned		
5	Hope High School	0	SW02	Upstream	SW01	Start node, manhole, reference: SW01		
5	Hope High School	0	SW02	Upstream	SW01	Water level, 90% of the vertical dimension		
5	Hope High School	0	SW02	Upstream	SW01	Loss of vision, camera under water, start		
5	Hope High School	5.99	SW02	Upstream	SW01	Loss of vision, camera under water, finish		
5	Hope High School	5.99	SW02	Upstream	SW01	Survey abandoned		



Project No.

Survey Date.

Surveyor.

Asset Survey Card

Asset ID: Asset Type: eg: MH, CP, GY Function: Cover Level:

Location:

Town:

COVER DETAILS

☒ Square ☐ Rectangle ☐ Triangle ☐ Dbl Triangle ☐ Circular ☐ Other Other

☐ Hinged ☐ Lockable ☐ Grating H Cover Duty x Size (mm)

PIPE DETAILS

Incoming Pipes

Pipe	Depth from Cover (m)	Upstream / Downstream Ref	Shape	Size (mm) Height Width		Pipe Mat	Lining Mat	Back Drop Dia (mm)	Invert Level (m)
A	3.14		C	150	x	VC			
B	3.2		C	150	x	VC			
C					x				
D					x				
E					x				
F					x				

Outgoing Pipes

X	3.21		C	150	x	VC			
Z					x				

MANHOLE CONSTRUCTION DETAILS

Shaft

x Size (mm) Depth (m) No. of Reg Courses Soffit

Chamber

x Size (mm) Side Entry OX / H2S / METH Toxic atmosphere Evidence of Surge

☒ Pre Cast Concrete ☐ Insitu Concrete ☐ Bolted Segments ☐ GRP ☐ Reducing Slab

Step Irons ☒ Ladder ☐ No. of Landings Depth of Flow (mm) Depth of Silt (mm)

ADDITIONAL CONSTRUCTION DETAILS

☐ Flap Valve ☐ Orifice ☐ Sluice Gate ☐ Weir ☐ Screens ☐ Penstock ☐ Flume ☐ H/Wall

CONDITION INFORMATION (Specify in Remark)

	Cover	Shaft	Irons/Ladder	Chamber	Benching	Channel
OK	☒	☒	☒	☒	☒	☒
Needs Attention	☐	☐	☐	☐	☐	☐
Urgent Attention	☐	☐	☐	☐	☐	☐

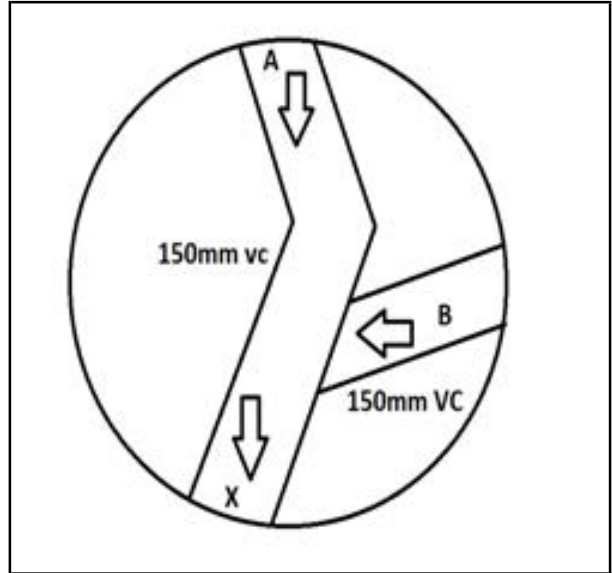
Remark

See Overleaf for Key

LOCATION SKETCH



PLAN OF MANHOLE



LOCATION PHOTO



INTERNAL PHOTO



STATUS	PU : Public	PR : Private	WC : Culverted Watercourse	} Applies to main outlet
	24 : Section 24	HD : Highway Drain		
FUNCTION	F : Foul	C : Combined	S : Surface Water	} Applies to main outlet
MANHOLE TYPE	IF : Infall	MH : Manhole	CP : Catchpit	
	LH : Lamphole	RE : Rodding Eye	SO : Storm Overflow	
	GU : Gully	OF : Outfall	SA : Soakaway	
COVER DUTY	L : Light Duty	M : Medium Duty	H : Heavy Duty	
SHAPE	C : Circular	E : Egg	R : Rectangular	
	S : Square	T : Trapezoidal	A : Arched	
	B : Barrel	H : Horseshoe (Inverted U)	U : U-shaped with flat top	
	X : Other (state in words)			
PIPE (OR LINING) MATERIAL	AK : Alkathene	AC : Asbestos Cement	BR : Brick	
	CI : Cast Iron	SI : Spun (Grey) Iron	CO : Concrete	
	CSB : Concrete Segments (Bolted)	CSU : Concrete Segments (Unbolted)	CC : Concrete Box Culvert	
	DI : Ductile Iron	GRC : Glass Reinforced Concrete	GRP : Glass Reinforced Plastic	
	PSC : Plastic/Steel Composite	PVC : Polyvinyl Chloride	PE : Polyethylene	
	RPM : Reinforced Plastic Matrix	ST : Steel	VC : Vitrified Clay	
	PP : Polypropylene	PF : Pitch Fibre	MAC : Masonry, Coursed	
	MAR : Masonry, Random Courses	XXX : Other (State in remarks)		
LINING	BL : Bitumen	RL : Resin	IS : Insituform	
	CL : Cement	PI : Plastic		



Project No. 134368
Survey Date. 01/11/2021
Surveyor. M Sweeney

Asset Survey Card

Asset ID: SW MH Asset Type: MH eg: MH, CP, GY Function: SW Cover Level:
Location: O/s 72 Becon Drive Skemesdale
Town: Liverpool

COVER DETAILS

☐ Square ☒ Rectangle ☐ Triangle ☐ Dbl Triangle ☐ Circular ☐ Other Double Lid Other
☐ Hinged ☐ Lockable ☐ Grating H Cover Duty 600 x 1200 Size (mm)

PIPE DETAILS

Incoming Pipes

Pipe	Depth from Cover (m)	Upstream / Downstream Ref	Shape	Size (mm) Height Width		Pipe Mat	Lining Mat	Back Drop Dia (mm)	Invert Level (m)
A	1.68		C	150	x	VC			
B	1.69		C	150	x	VC			
C					x				
D					x				
E					x				
F					x				

Outgoing Pipes

X	1.71		C	150	x	VC			
Z					x				

MANHOLE CONSTRUCTION DETAILS

Shaft

600 x 1200 Size (mm) 58 Depth (m) 3 No. of Reg Courses Soffit

Chamber

1200 x Size (mm) Y Side Entry N OX / H2S / METH Toxic atmosphere N Evidence of Surge

☒ Pre Cast Concrete ☐ Insitu Concrete ☐ Bolted Segments ☐ GRP ☐ Reducing Slab
3 Step Irons ☒ Ladder ☐ No. of Landings 10 Depth of Flow (mm) 10 Depth of Silt (mm)

ADDITIONAL CONSTRUCTION DETAILS

☐ Flap Valve ☐ Orifice ☐ Sluice Gate ☐ Weir ☐ Screens ☐ Penstock ☐ Flume ☐ H/Wall

CONDITION INFORMATION (Specify in Remark)

	Cover	Shaft	Irons/Ladder	Chamber	Benching	Channel
OK	☒	☒	☒	☒	☒	☒
Needs Attention	☐	☐	☐	☐	☐	☐
Urgent Attention	☐	☐	☐	☐	☐	☐

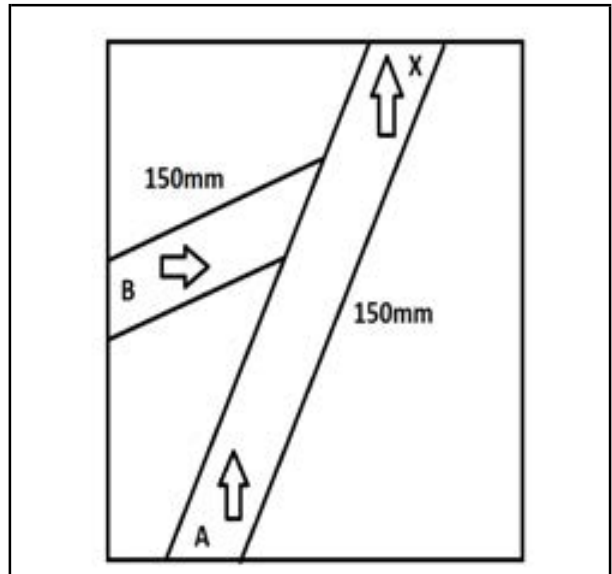
Remark

See Overleaf for Key

LOCATION SKETCH



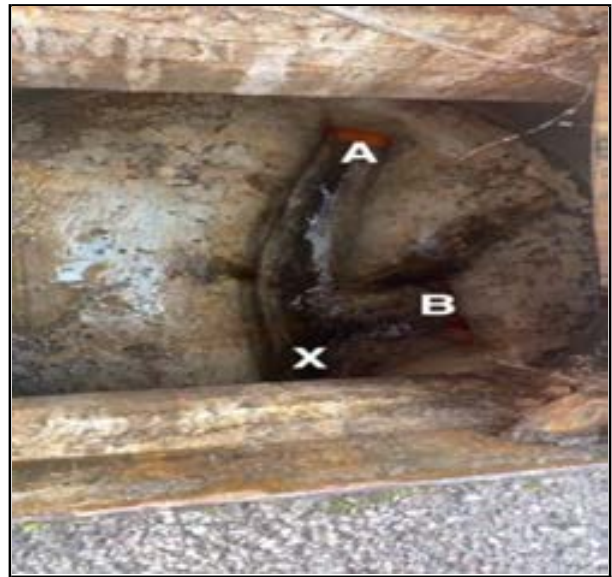
PLAN OF MANHOLE



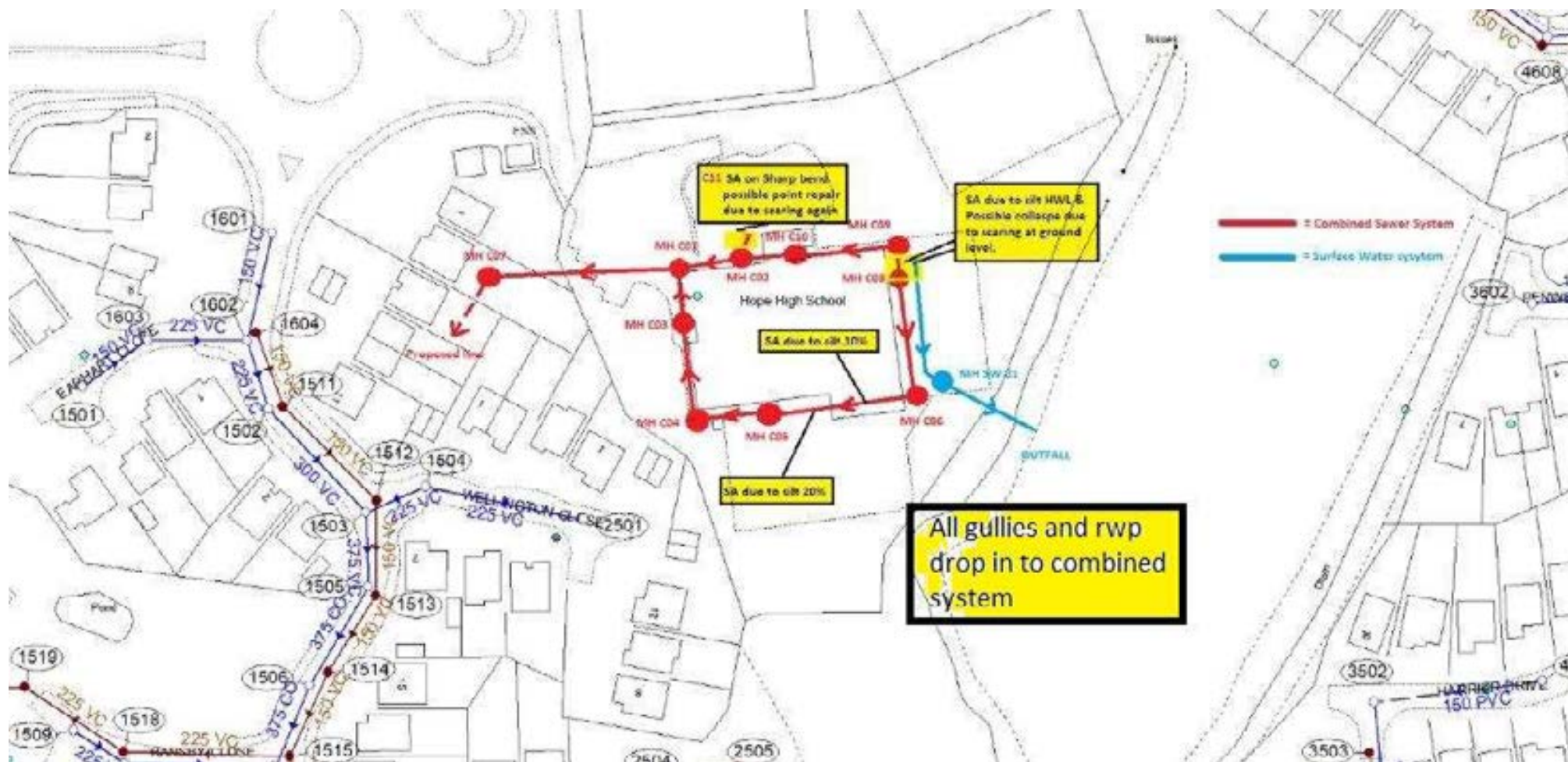
LOCATION PHOTO

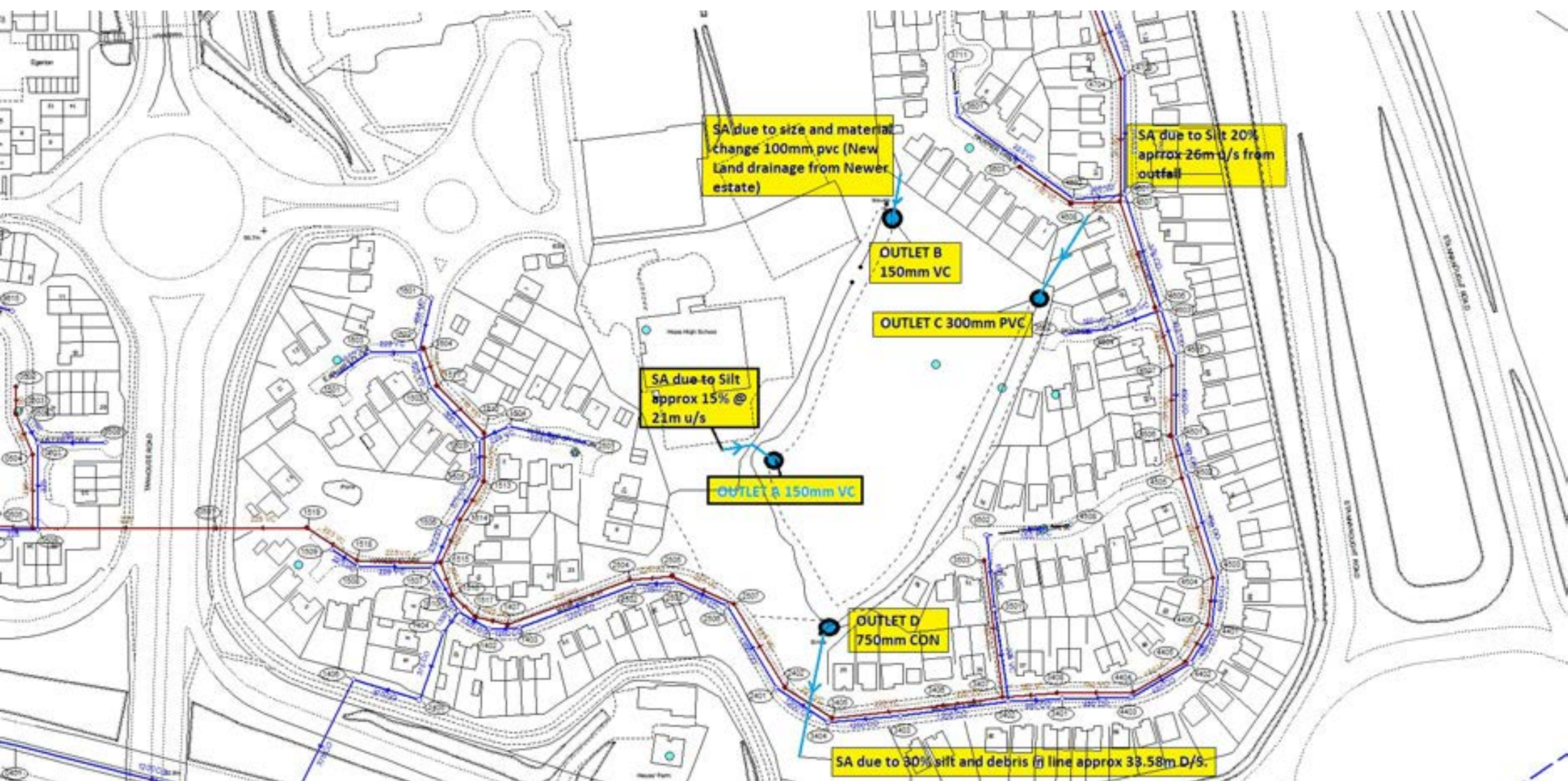


INTERNAL PHOTO



STATUS	PU : Public	PR : Private	WC : Culverted Watercourse	} Applies to main outlet
	24 : Section 24	HD : Highway Drain		
FUNCTION	F : Foul	C : Combined	S : Surface Water	} Applies to main outlet
MANHOLE TYPE	IF : Infall	MH : Manhole	CP : Catchpit	
	LH : Lamphole	RE : Rodding Eye	SO : Storm Overflow	
	GU : Gully	OF : Outfall	SA : Soakaway	
COVER DUTY	L : Light Duty	M : Medium Duty	H : Heavy Duty	
SHAPE	C : Circular	E : Egg	R : Rectangular	
	S : Square	T : Trapezoidal	A : Arched	
	B : Barrel	H : Horseshoe (Inverted U)	U : U-shaped with flat top	
	X : Other (state in words)			
PIPE (OR LINING) MATERIAL	AK : Alkathene	AC : Asbestos Cement	BR : Brick	
	CI : Cast Iron	SI : Spun (Grey) Iron	CO : Concrete	
	CSB : Concrete Segments (Bolted)	CSU : Concrete Segments (Unbolted)	CC : Concrete Box Culvert	
	DI : Ductile Iron	GRC : Glass Reinforced Concrete	GRP : Glass Reinforced Plastic	
	PSC : Plastic/Steel Composite	PVC : Polyvinyl Chloride	PE : Polyethylene	
	RPM : Reinforced Plastic Matrix	ST : Steel	VC : Vitrified Clay	
	PP : Polypropylene	PF : Pitch Fibre	MAC : Masonry, Coursed	
	MAR : Masonry, Random Courses	XXX : Other (State in remarks)		
LINING	BL : Bitumen	RL : Resin	IS : Insituform	
	CL : Cement	PI : Plastic		





APPENDIX D

ENVIRONMENT AGENCY CONSULTATION

Matthew Wilson

From: CMBLNC Info Requests <Inforequests.cmblnc@environment-agency.gov.uk>
Sent: Tuesday 21 January, 2020 2:27 pm
To: Matthew Wilson
Subject: CL157879BL

Follow Up Flag: Follow up
Flag Status: Completed

Dear Matthew

This site is located in Flood Zone 1, meaning the estimated risk of flooding from rivers/sea is less than 0.1% (1 in 1000) in any given year. We therefore do not have any relevant modelling information to provide for a product 4. For further information on flood zones, please visit the Flood Map for Planning on the gov.uk website: <https://flood-map-for-planning.service.gov.uk/>.

The Environment Agency has no record of previous flooding in this area.

For further guidance on producing a flood risk assessment in Flood Zone 1, please see the link below.
<https://www.gov.uk/guidance/flood-risk-assessment-in-flood-zone-1-and-critical-drainage-areas>

Surface Water Maps can be viewed online at <https://flood-warning-information.service.gov.uk/long-term-flood-risk/map> or can be downloaded using the data.gov.uk Web Map Service. For further information relating to the risk of flooding from surface water, please contact the Lead Local Flood Authority Lancashire County Council.

Thanks

Becca

Rebecca Lettice

Customers and Engagement Officer, Cumbria and Lancashire

Environment Agency | Ghyll Mount, Gillan Way, Penrith 40 Business Park, Penrith, Cumbria, CA11 9BP

inforequests.cmblnc@environment-agency.gov.uk



From: Enquiries, Unit
Sent: 21 January 2020 13:07
To: MatthewWilson@arnorthwest.co.uk
Subject: REF: 200121/DJ09. NW113-03-02 Former Hope High School, Skelmersdale - Flood risk enquiry

Dear Matthew

I have passed your e-mail to the local customer team who will deal with your request.

The Freedom of Information Act and Environmental Information Regulations state that a public authority must respond to requests for information within 20 working days, although we aim to respond to all enquiries as quickly as we can.

You can find more information about our service commitment by clicking on the link below:

<https://www.gov.uk/government/publications/environment-agency-customer-service-commitment>

You can contact our customer team directly on the contact details below, or call the National Customer Contact Centre on 03708 506506 who will transfer you to the area team.

Please quote your enquiry reference 200121/DJ09 in any correspondence with us regarding this matter.

Customers and Engagement
Environment Agency
Cumbria and Lancashire Area
Lutra House
Dodd Way
Walton Summit
Bamber Bridge
PRESTON
PR5 8BX

Kind regards

David Jackson
Customer Service Advisor
National Customer Contact Centre
Environment Agency

☎ Tel: 03708 506 506

💻 www.gov.uk/environment-agency

Click an icon to keep in touch with us:-



From: Matthew Wilson [<mailto:MatthewWilson@arnorthwest.co.uk>]

Sent: 20 January 2020 15:59

To: Enquiries, Unit <enquiries@environment-agency.gov.uk>

Subject: NW113-03-02 Former Hope High School, Skelmersdale - Flood risk enquiry

Good afternoon,

We have been instructed to prepare a Flood Risk Assessment and Drainage Strategy to support a planning application for a proposed residential development at the former Hope High School, Carfield, Clay Brow, Skelmersdale WN8 9DP.

The proposed development site is centred on approximate Ordnance Survey grid reference 350301, 405612, and the site is approximately 2.97ha in area. I have attached a location plan for reference.

The site is a former school, with an existing building and areas of existing hardstanding in the west, plus undeveloped greenfield land in the north and east. The proposed development is for circa 50 - 70 residential properties.

Having reviewed the EA Flood Map for Planning we anticipate that the site falls within Flood Zone 1. We would appreciate your confirmation of this, and provision of any relevant 'Product 4' data for the site.

Ordnance Survey mapping indicates two small watercourses/drains located in the east. There is an unnamed watercourse approximately 850m north-west of the site, which is a tributary of the River Tawd (which is located further west). We would appreciate if you could provide any information on the connectivity of the drains within the site, or any information on any other known watercourses in the vicinity which may be suitable for surface water discharge?

To inform the Flood Risk Assessment, we would also appreciate your advice on any known flooding within or near to the site, or any other issues which may be pertinent to our work.

For reference, we are also in consultation with United Utilities and the Lead Local Flood Authority regarding surface water management requirements/options.

We trust that this is satisfactory and look forward to hearing from you, however, should you require any additional information at this time please do not hesitate to contact us.

Kind regards,

Matthew Wilson
Principal Engineer

07307 849 396

matthewwilson@arpnorthwest.co.uk



www.arpassociates.co.uk

2nd Floor, 3 Hardman Square, Spinningfields, Manchester, M3 3EB
5 & 6 Northwest Business Park, Servia Hill, Leeds LS6 2QH

Pre-planning Geotechnical Civil Engineering Structural Engineering Building Services Engineering

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Flood map for planning

Your reference
Hope High

Location (easting/northing)
350306/405600

Created
6 May 2022 0:24

Your selected location is in flood zone 1, an area with a low probability of flooding.

You will need to do a flood risk assessment if your site is **any of the following:**

- bigger than 1 hectare (ha)
- In an area with critical drainage problems as notified by the Environment Agency
- identified as being at increased flood risk in future by the local authority's strategic flood risk assessment
- at risk from other sources of flooding (such as surface water or reservoirs) and its development would increase the vulnerability of its use (such as constructing an office on an undeveloped site or converting a shop to a dwelling)

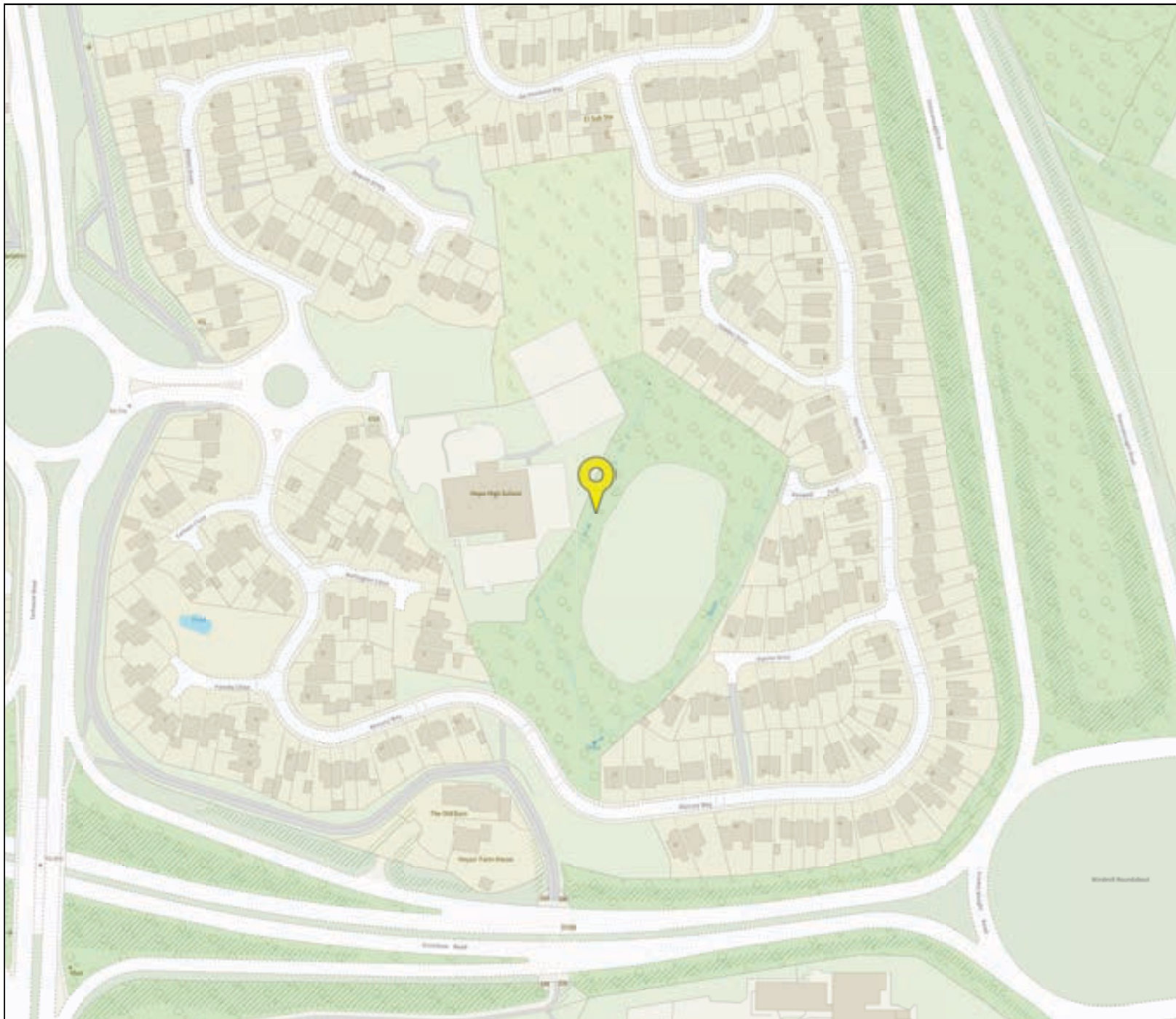
Notes

The flood map for planning shows river and sea flooding data only. It doesn't include other sources of flooding. It is for use in development planning and flood risk assessments.

This information relates to the selected location and is not specific to any property within it. The map is updated regularly and is correct at the time of printing.

Flood risk data is covered by the Open Government Licence **which** sets out the terms and conditions for using government data. <https://www.nationalarchives.gov.uk/doc/open-government-licence/version/3/>

Use of the address and mapping data is subject to Ordnance Survey public viewing terms under Crown copyright and database rights 2021 OS 100024198. <https://flood-map-for-planning.service.gov.uk/os-terms>



Flood map for planning

Your reference

Hope High

Location (easting/northing)

350306/405600

Scale

1:2500

Created

6 May 2022 0:24

-  Selected point
-  Flood zone 3
-  Flood zone 3: areas benefiting from flood defences
-  Flood zone 2
-  Flood zone 1
-  Flood defence
-  Main river
-  Water storage area

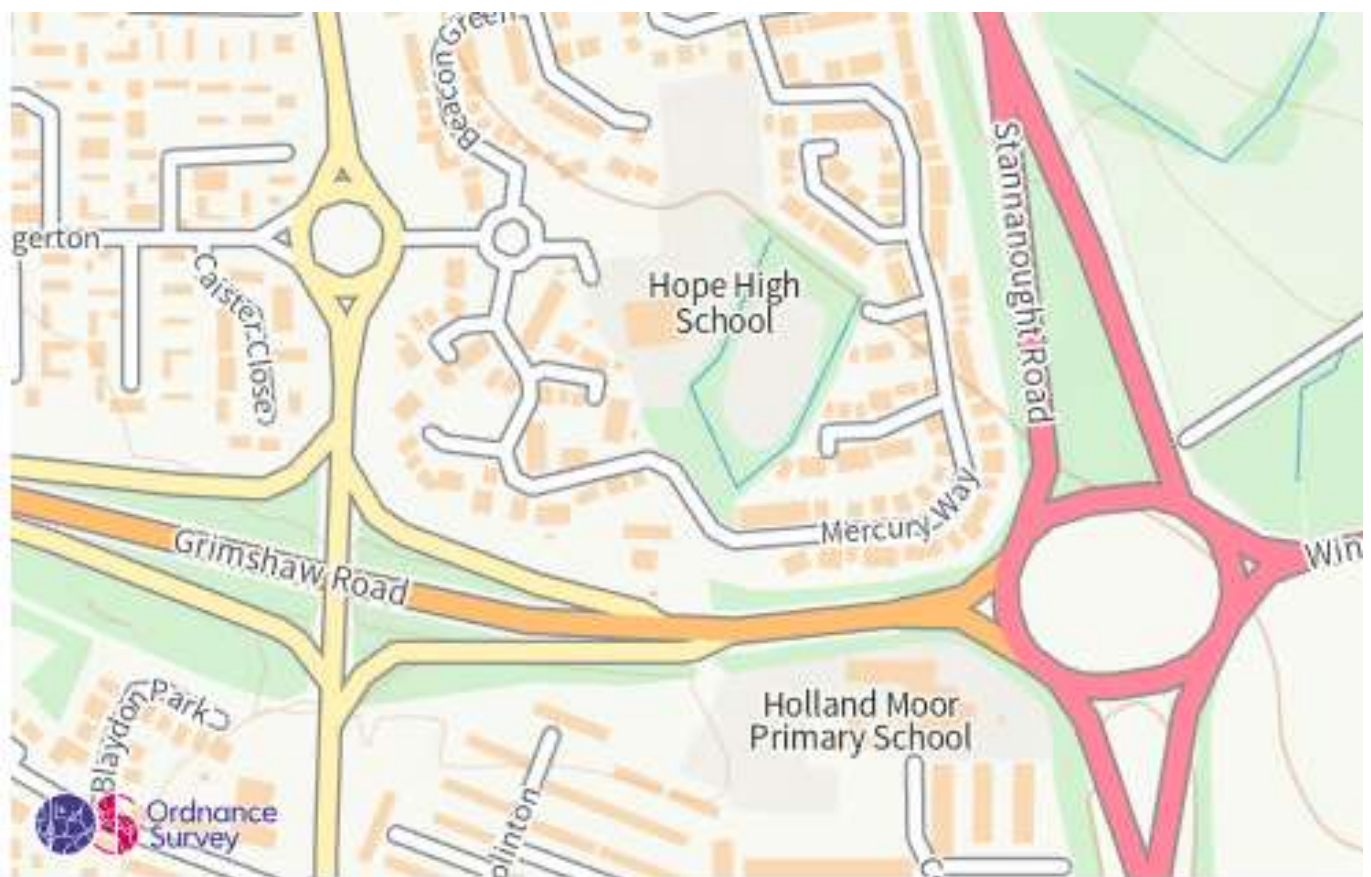
0 20 40 60m

Extract from Environment Agency Risk of Flooding from Surface Water Map



Extent of flooding from surface water





Maximum extent of flooding from reservoirs:

- when river levels are normal
- when there is also flooding from rivers

APPENDIX E

WATER AUTHORITY CONSULTATION

Matthew Wilson

From: Dodd, Matthew <Matthew.Dodd@uuplc.co.uk>
Sent: Thursday 20 February, 2020 2:46 pm
To: Roy Fowler; Wastewater Developer Services
Cc: Matthew Wilson
Subject: RE: NW134/01/2 Former Hope High School, Skelmersdale WN8 6BW - PDE 4200030297

Good Afternoon Roy

We have carried out an assessment of your application which is based on the information provided; this pre development advice will be valid for 12 months

Foul

Foul will be allowed to drain to the public foul sewer network at an unrestricted rate.

Surface Water

Surface water from this site must discharge to ground (via infiltration) using a soak away or some other form of infiltration system. If ground investigation/ ground conditions confirm that infiltration is not a viable solution, and there is not an available waterbody, all surface water can drain to the surface water sewer network at a pass forward flow to be agreed by the Lead Local Flood Authority. United Utilities request that any agreed rate does not exceed 15 l/s.

Should your proposals to discharge to the public sewer include highway drainage runoff as well as surface water runoff from the buildings and yards, United Utilities request the inclusion of SuDS components that will control the volume and rate of discharge to the receiving sewer to be demonstrated within the sites drainage strategy.

Connection Application

Although we may discuss and agree discharge points & rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimately inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below

<http://www.unitedutilities.com/connecting-public-sewer.aspx>

Sewer Adoption Agreement

You may wish to offer the proposed new sewers for adoption. United Utilities assess adoption application based on Sewers adoption 6th Edition and for any pumping stations our company addenda document. Please refer to link below to obtain further guidance and application pack:

<http://www.unitedutilities.com/sewer-adoption.aspx>

Please be aware that on site drainage must be designed in accordance with Building Regulations, National Planning Policy, and local flood authority guidelines, we would recommend that you speak and make suitable agreements with the relevant statutory bodies.

Please note, if you intend to put forward your wastewater assets for adoption by United Utilities, the proposed detail design will be subject to a technical appraisal by an Adoption Engineer as we need to be sure that the proposals meets the requirements of Sewers for adoption and United Utilities Asset Standards. The proposed design should give

consideration to long term operability and give United Utilities a cost effective proposal for the life of the assets. Therefore, further to this enquiry should you wish to progress a Section 104 agreement, we strongly recommend that no construction commences until the detailed drainage design, submitted as part of the Section 104 agreement, has been assessed and accepted in writing by United Utilities. Any works carried out prior to the technical assessment being approved is done entirely at the developers own risk and could be subject to change.

Regards



Matthew Dodd
Developer Engineer
Developer Services & Metering
Customer Services
T: 01925 679369
unitedutilities.com

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Visit: unitedutilities.com/wow

From: Roy Fowler [mailto:RoyFowler@arnorthwest.co.uk]
Sent: 11 February 2020 13:02
To: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Cc: Matthew Wilson <MatthewWilson@arnorthwest.co.uk>
Subject: NW134/01/2 Former Hope High School, Skelmersdale WN8 6BW - PDE 4200030297

Sirs,

Please find attached a Wastewater pre-development for the above site, with a Site Location plan and a proposed Development Plan.

We would appreciate your comments, especially any allowable surface water discharge rate and surface water and foul water connection points into existing UU sewers.

Soakaways are not thought to be practical on this site due to the cohesive nature of the ground.

If you have any queries, please do not hesitate to contact us.

Regards

Roy Fowler BEng (Hons)
Senior Engineer

DD 0161 914 9671

royfowler@arnorthwest.co.uk



Chartered Consulting Engineers

2nd Floor, 3 Hardman Square, Spinningfields, Manchester, M3 3EB

Pre-planning Geotechnical Civil Engineering Structural Engineering Building Services Engineering

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EMGateway3.uuplc.co.uk made the following annotations

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www.unitedutilities.com/subsidiaries

ARP (North West) Ltd

**5/6 Northwest Business Park
Servia Hill,
Leeds,
LS6 2QH**

FAO:

How to contact us:

**United Utilities Water Limited
Property Searches
Haweswater House
Lingley Mere Business Park
Great Sankey
Warrington
WA5 3LP**

Telephone: 0370 7510101

E-mail: propertysearches@uuplc.co.uk

**Your Ref: NW113-03-02
Our Ref: UUPS-ORD-144250
Date: 07/01/2020**

Dear Sirs

Location: 2 WELLINGTON CLOSE, SKELMERSDALE, WN8 6BJ

I acknowledge with thanks your request dated 07/01/2020 for information on the location of our services.

Please find enclosed plans showing the approximate position of United Utilities' apparatus known to be in the vicinity of this site.

The enclosed plans are being provided to you subject to the United Utilities terms and conditions for both the wastewater and water distribution plans which are shown attached.

If you are planning works anywhere in the North West, please read United Utilities' access statement before you start work to check how it will affect our network. <http://www.unitedutilities.com/work-near-asset.aspx>.

I trust the above meets with your requirements and look forward to hearing from you should you need anything further.

If you have any queries regarding this matter please [contact us](#).

Yours Faithfully,



Karen McCormack
Property Searches Manager

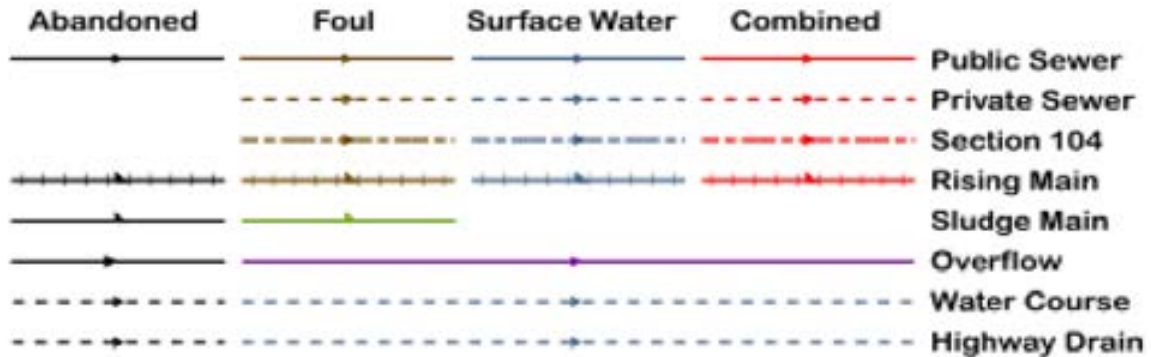
TERMS AND CONDITIONS - WASTEWATER AND WATER DISTRIBUTION PLANS

These provisions apply to the public sewerage, water distribution and telemetry systems (including sewers which are the subject of an agreement under Section 104 of the Water Industry Act 1991 and mains installed in accordance with the agreement for the self construction of water mains) (UUWL apparatus) of United Utilities Water Limited "(UUWL)".

TERMS AND CONDITIONS:

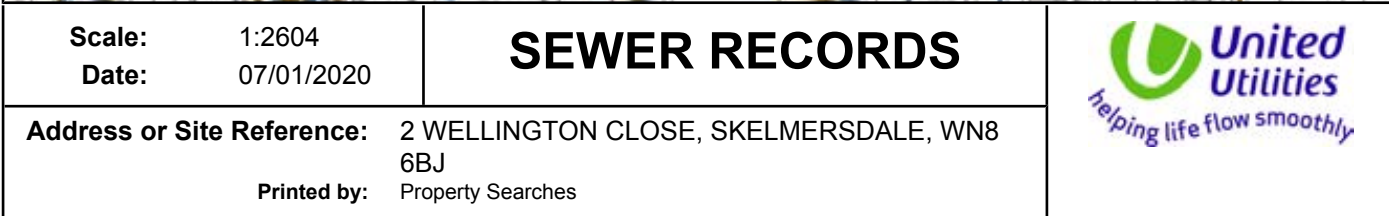
- This Map and any information supplied with it is issued subject to the provisions contained below, to the exclusion of all others and no party relies upon any representation, warranty, collateral contract or other assurance of any person (whether party to this agreement or not) that is not set out in this agreement or the documents referred to in it.
- This Map and any information supplied with it is provided for general guidance only and no representation, undertaking or warranty as to its accuracy, completeness or being up to date is given or implied.
- In particular, the position and depth of any UUWL apparatus shown on the Map are approximate only. UUWL strongly recommends that a comprehensive survey is undertaken in addition to reviewing this Map to determine and ensure the precise location of any UUWL apparatus. The exact location, positions and depths should be obtained by excavation trial holes.
- The location and position of private drains, private sewers and service pipes to properties are not normally shown on this Map but their presence must be anticipated and accounted for and you are strongly advised to carry out your own further enquiries and investigations in order to locate the same.
- The position and depth of UUWL apparatus is subject to change and therefore this Map is issued subject to any removal or change in location of the same. The onus is entirely upon you to confirm whether any changes to the Map have been made subsequent to issue and prior to any works being carried out.
- This Map and any information shown on it or provided with it must not be relied upon in the event of any development, construction or other works (including but not limited to any excavations) in the vicinity of UUWL apparatus or for the purpose of determining the suitability of a point of connection to the sewerage or other distribution systems.
- No person or legal entity, including any company shall be relieved from any liability howsoever and whensoever arising for any damage caused to UUWL apparatus by reason of the actual position and/or depths of UUWL apparatus being different from those shown on the Map and any information supplied with it.
- If any provision contained herein is or becomes legally invalid or unenforceable, it will be taken to be severed from the remaining provisions which shall be unaffected and continue in full force and affect.
- This agreement shall be governed by English law and all parties submit to the exclusive jurisdiction of the English courts, save that nothing will prevent UUWL from bringing proceedings in any other competent jurisdiction, whether concurrently or otherwise.

Wastewater Symbolology



All point assets follow the standard colour convention: **red** – combined **brown** - foul
blue – surface water **purple** - overflow

- | | |
|-------------------------|---------------------------------|
| Manhole | Side Entry Manhole |
| Head of System | Outfall |
| Extent of Survey | Screen Chamber |
| Rodding Eye | Inspection Chamber |
| Inlet | Bifurcation Chamber |
| Discharge Point | Lamp Hole |
| Vortex | T Junction / Saddle |
| Penstock | Catchpit |
| Washout Chamber | Valve Chamber |
| Valve | Vent Column |
| Air Valve | Vortex Chamber |
| Non Return Valve | Penstock Chamber |
| Soakaway | Network Storage Tank |
| Gully | Sewer Overflow |
| Cascade | Ww Treatment Works |
| Flow Meter | Ww Pumping Station |
| Hatch Box | Septic Tank |
| Oil Interceptor | Control Kiosk |
| Summit | |
| Drop Shaft | Change of Characteristic |
| Orifice Plate | |



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Clean Water Symbolology

Proposed	Abandoned	Live	
			Distribution Main
			Trunk Main
			Comms Pipe
			Private Pipe
			Concessionary Service
			Raw Water
			LDTM Raw Water
			LDTM Treated Water
			Air Valve
			AC Valve, open
			AC Valve, closed
			CC Valve, open
			CC Valve, closed
			Non Return Valve
			Pressure Management Valve
			OMS Valve
			Stop Tap
			Flow Meter
			Domestic Meter
			Commercial Meter
			Pump
			Hydrant
			Fire Hydrant
			Anode
			Chlorination Point
			De-chlorination Point
			Strainer Point
			Access Point
			Hatch Box
			IP Point
			Sampling Station
			Logger Box
			Bore Hole
			Inlet Point
			Bulk Supply Point
			End Cap
			Site Termination
			Change of Characteristic
			Condition Report
			<u>Property Types</u>
			Water Tower
			Valve House
			Booster Pumping Station
			Intake Pumping Station
			Water Treatment Works
			Supply Reservoir
			Service Reservoir
			Impounding Reservoir
			Pipe Bridge

Symbology for proposed assets is the same as above, but shown in **green**
Symbology for abandoned assets is the same as above, but shown in **black**



Scale: 1:2604
Date: 07/01/2020

WATER MAINS RECORDS

Address or Site Reference: 2 WELLINGTON CLOSE, SKELMERSDALE, WN8 6BJ

Printed by: Property Searches

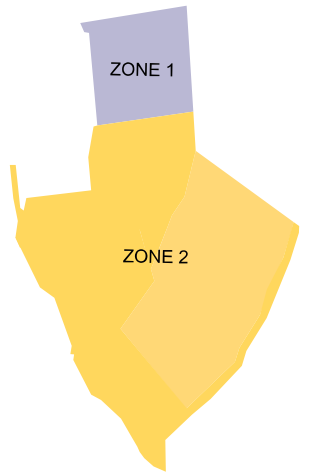
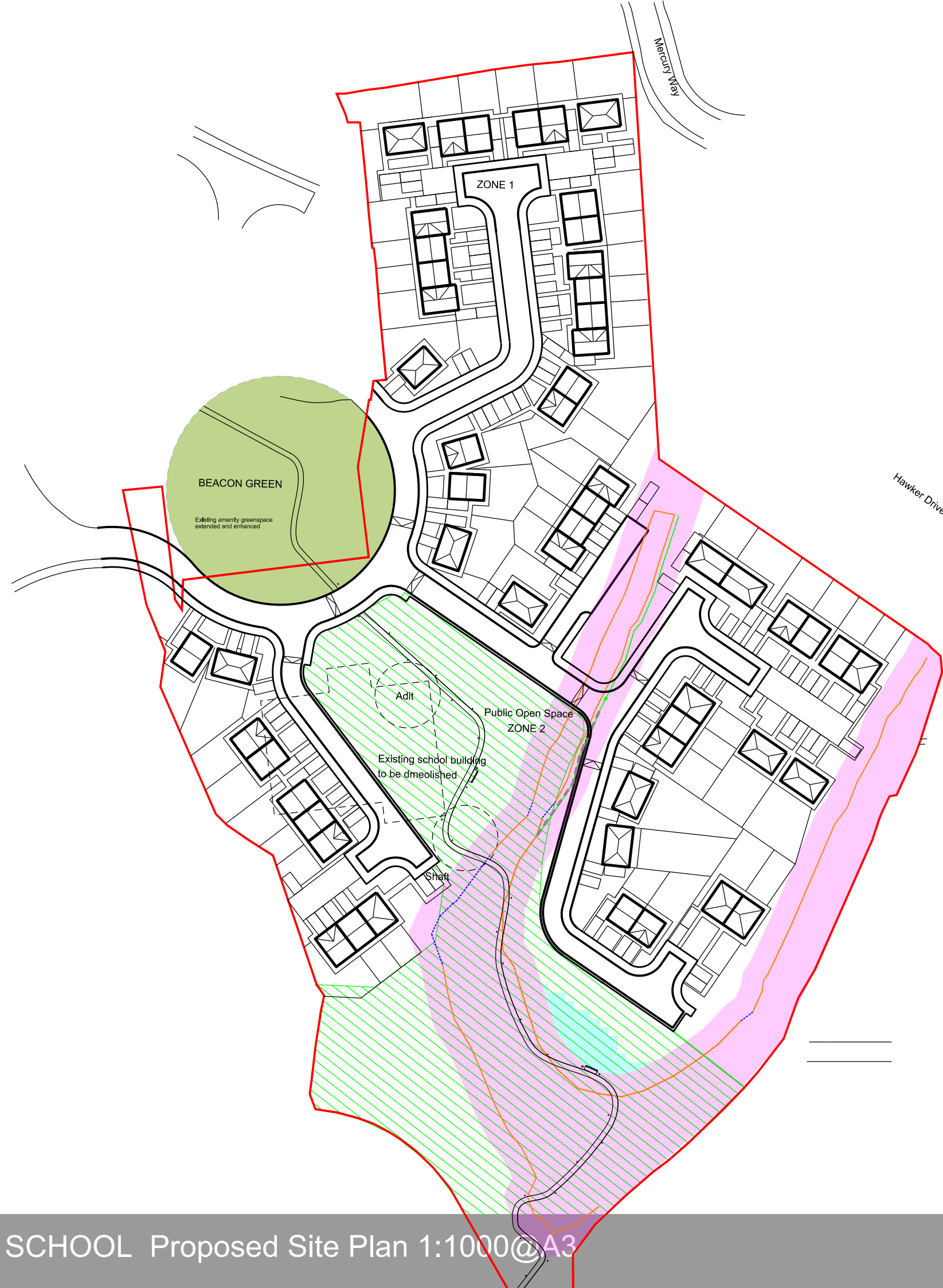


The position of the underground apparatus shown on this plan is approximate only and is given in accordance with the best information currently available. United Utilities Water will not accept liability for any loss or damage caused by the actual position being different from those shown.

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APPENDIX F

DEVELOPMENT PROPOSALS



Schedule of accommodation

2B4P House - 79m ²	10No.
3B5P House standard - 93m ²	28No.
3B5P House gable fronted - 93m ²	4No.
4B6P House detached - 106m ²	10No.
4B6P House semi - 106m ²	2No.
Total	54No.

Note:
All house types are designed to 100% NDSS
Car parking allocation to suit WLBC Planning policy and LCC guidance:
200% to 2 and 3 beds, 300% to 4 beds

Public open space requirements:
(Number of bedrooms 115No.)

Zone 1 - Designated as public open space
~4,580m²

Public open space required for new build units
2,025m²

Total Public Open Space required
8,120m²



APPENDIX G

GREENFIELD RUNOFF CALCULATION

Calculated by:	Matthew Wilson
Site name:	Hope
Site location:	Skelmersdale

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Site Details

Latitude:	53.54534° N
Longitude:	2.75193° W
Reference:	2372446158
Date:	Apr 03 2020 17:13

Runoff estimation approach

IH124

Site characteristics

Total site area (ha):	1
-----------------------	---

Methodology

Q _{BAR} estimation method:	Calculate from SPR and SAAR
SPR estimation method:	Calculate from SOIL type

Soil characteristics

	Default	Edited
SOIL type:	4	4
HOST class:	N/A	N/A
SPR/SPRHOST:	0.47	0.47

Hydrological characteristics

	Default	Edited
SAAR (mm):	964	964
Hydrological region:	10	10
Growth curve factor 1 year:	0.87	0.87
Growth curve factor 30 years:	1.7	1.7
Growth curve factor 100 years:	2.08	2.08
Growth curve factor 200 years:	2.37	2.37

Notes

(1) Is Q_{BAR} < 2.0 l/s/ha?

When Q_{BAR} is < 2.0 l/s/ha then limiting discharge rates are set at 2.0 l/s/ha.

(2) Are flow rates < 5.0 l/s?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

(3) Is SPR/SPRHOST ≤ 0.3?

Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

Greenfield runoff rates

	Default	Edited
Q _{BAR} (l/s):	7.02	7.02
1 in 1 year (l/s):	6.11	6.11
1 in 30 years (l/s):	11.93	11.93
1 in 100 year (l/s):	14.6	14.6
1 in 200 years (l/s):	16.64	16.64

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.